



## Appendix D – Catwalk Calculations

### Steel Catwalk Load Calculations:

Assumptions:

- Considering HSS 5x5x5/16 hangers and W8x28 girders are the critical members
- The catwalk is 1' wide, with largest spans of 25'
- Load requirements are 40 PSF LL and 20 PSF DL

HSS 5x5x5/16 hanger –

$$\text{Tributary Area} = 25' \times 1' = 25 \text{ ft}^2$$

$$1.2(20 \text{ PSF}) + 1.6(40 \text{ PSF}) = 88 \text{ PSF}$$

$$25 \text{ ft}^2 \times 88 \text{ PSF} = 2,000 \text{ lb}$$

$$\text{Stress} = P/A = 2.2 \text{ kips} / 8.42 \text{ in}^2 = 0.27 \text{ ksi} < 50 \text{ ksi}$$

W8x28 girder –

$$W = 88 \text{ PSF} \times 1' = 88 \text{ PLF}$$

$$V_u = (wl)/2 = (88 \text{ PLF} \times 25 \text{ ft})/2 = 1,100 \text{ lbs}$$

$$M_u = (wl^2)/8 = [8 \text{ PLF} \times (25\text{ft})^2]/8 = 6,875 \text{ ft-lbs}$$

$$\text{*DL \& LL: } \Delta = (5wl^4)/384EI$$

$$= [5 \times 88 \text{ PLF} \times (25\text{ft})^4 \times 1728 \text{ in}^3] / (384 \times 29\text{e}3 \text{ ksi} \times 98 \text{ in}^4 \times 1,000 \text{ lbs}) = 0.272 \text{ in}$$

$$0.272 \text{ in} < 0.625 = (25 \text{ ft} \times 12 \text{ in/ft}) / 480$$

$$\text{*LL: } \Delta = (5wl^4)/384EI$$

$$= 5 \times 64 \text{ PLF} \times (25\text{ft})^4 \times 1728 \text{ in}^3 / (384 \times 29\text{e}3 \text{ ksi} \times 98 \text{ in}^4 \times 1,000 \text{ lbs}) = 0.198 \text{ in}$$

$$0.198 \text{ in} < 0.833 = (25 \text{ ft} \times 12 \text{ in/ft}) / 360$$

$$Z_{\text{required}} = M_u / \Phi_b F_y = (6,875 \text{ ft-lbs} \times 12 \text{ in}) / (0.9 \times 50 \text{ ksi} \times 1,000\text{lbs}) = 1.83 \text{ in}^3$$



### Aluminum Catwalk Load Calculations:

Assumptions:

- Considering HSS 4x4x3/16 hangers and W10x210 girders are the critical members
- The catwalk is 1' wide, with largest spans of 25'
- Load requirements are 40 PSF LL and 20 PSF DL
- $F_y = 35$  ksi and  $E = 10e3$  ksi for alloy 6061-T6
- Additional material characteristics are to be that of steel, allowing for the same equations

HSS 4x4x3/16 hanger –

$$\text{Stress} = P/A = 2.2 \text{ kips} / 2.87 \text{ in}^2 = 0.77 \text{ ksi} < 35 \text{ ksi}$$

W8x28 girder –

$$W = 88 \text{ PSF} \times 1' = 88 \text{ PLF}$$

$$V_u = (wl)/2 = (88 \text{ PLF} \times 25 \text{ ft})/2 = 1,100 \text{ lbs}$$

$$M_u = (wl^2)/8 = [8 \text{ PLF} \times (25\text{ft})^2]/8 = 6,875 \text{ ft-lbs}$$

$$\text{*DL \& LL: } \Delta = (5wl^4)/384EI$$

$$= [5 \times 88 \text{ PLF} \times (25 \text{ ft})^4 \times 1728 \text{ in}^3] / (384 \times 10e3 \text{ ksi} \times 155.8 \text{ in}^4 \times 1,000 \text{ lbs}) = 0.496 \text{ in}$$

$$0.496 \text{ in} < 0.625 = (25 \text{ ft} \times 12 \text{ in/ft}) / 480$$

$$\text{*LL: } \Delta = (5wl^4)/384EI$$

$$= [5 \times 64 \text{ PLF} \times (25 \text{ ft})^4 \times 1728 \text{ in}^3] / (384 \times 10e3 \text{ ksi} \times 155.8 \text{ in}^4 \times 1,000 \text{ lbs}) = 0.198 \text{ in}$$

$$0.37 \text{ in} < 0.833 = (25 \text{ ft} \times 12 \text{ in/ft}) / 360$$

### FRP Catwalk Load Calculations:

No calculations were evaluated for this section. E.T. Techtonics estimator considered the 40 PSF live load and 20 PSF deal load.

### Wood Catwalk Load Calculations:

No calculations were evaluated for this section. The steel hangers remained in this design and have already been checked. Manufactured I-beams were recommended by a Georgia-Pacific Product Guide.