



FDA CDRH Laboratory

Timothy Mueller

Senior Thesis, Spring 2006

Structural Option



FDA CDRH Laboratory

Timothy Mueller • Senior Thesis • Spring 2006 • Structural Option

Project Background

General Architecture

Existing Structure

Depth Study

- Gravity Analysis

- Lateral Analysis

- Additional Considerations

Breadth Study

- Construction Management

- Architectural Analysis

Summary and Conclusions

Acknowledgements



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Navy Ordnance Site



U.S. General Services Administration



U.S. Food and Drug Administration



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Delivery Method: Design-Bid-Build

Major Building Code: IBC 2000

Cost: \$63 Million

Start Date: March 22, 2005

Finish Date: November 1, 2006



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GENERAL ARCHITECTURE



Façade:

- Many decorative aluminum & sheet metal panels
- Ribbon windows
- Full glazing curtain walls
- Horizontal sunshields

High Bay Laboratory:

- Located on West Side
- Decorative curved metal roof

Size:

- 139,805 Sq Ft

Height:

- 86' above grade
- Central core w/ 5th floor penthouse
- Four story main structure
- One floor below grade





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EXISTING STRUCTURE

Roof:

Typical concrete on metal deck w/ steel frame of:

- W14X122
- W10X73

Superstructure:

Typically one-way cast-in-place concrete w/ monolithic poured:

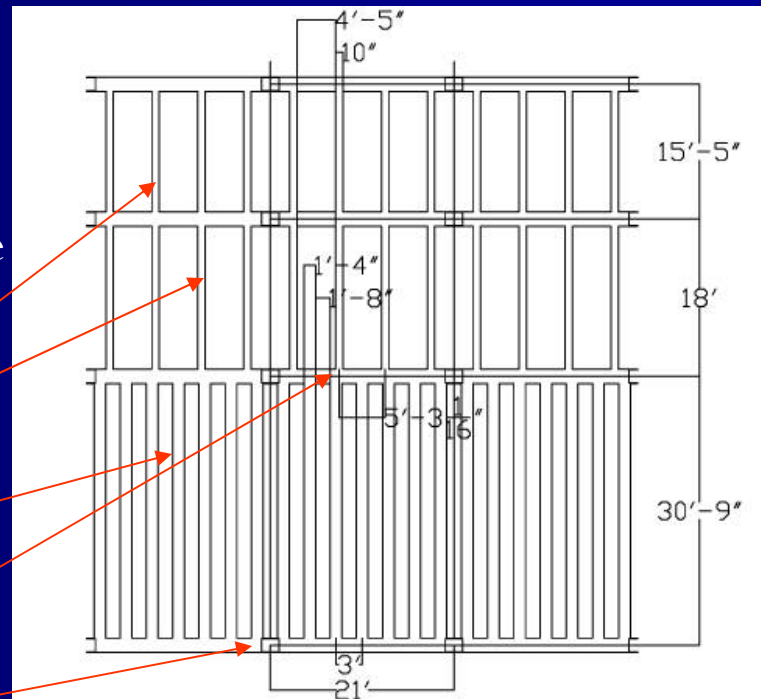
- 4.5" slab
- 10"X16" joist
- 16"X16" joist
- 20"X20.5" beams
- 18"X24" columns

Unique protection:

- 20"X30" progressive collapse beams

Foundation:

- 3' deep step footing
- 10'X10' spread footing below columns





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Gravity Analysis

Lateral Analysis

Additional Considerations

Breadth Study

Construction Management

Architectural Analysis

Summary and Conclusions

Acknowledgements



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Considerations:

Concrete Pros:

- High Vibration Stability
- Integrated Fireproofing
- Small Floor Sandwich

Concrete Cons:

- Labor Intensive
- Large Total Mass
- Steel Roof System

Proposed Solution:

- Construct the FDA CDRH Laboratory with Steel



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Loading:

Dead load: 73psf

USF2X deck and Concrete: 48psf

Superimposed: 25psf

Snow load (Washington D.C.): 30psf

Live Load: 125psf

Light Manufacturing (Most Laboratory Spaces): 125psf

Light Storage (Supplementary Laboratory Spaces): 125psf

The controlling combination in both N/S and E/W direction is

$$1.2D + 1.0E + 0.5L + 0.2S$$

for all floors except the first floor which was controlled in both directions by

$$1.2D + 1.6W + 0.5L + 0.5S$$



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Lateral Analysis

Additional Considerations

Breadth Study

Construction Management

Architectural Analysis

Summary and Conclusions

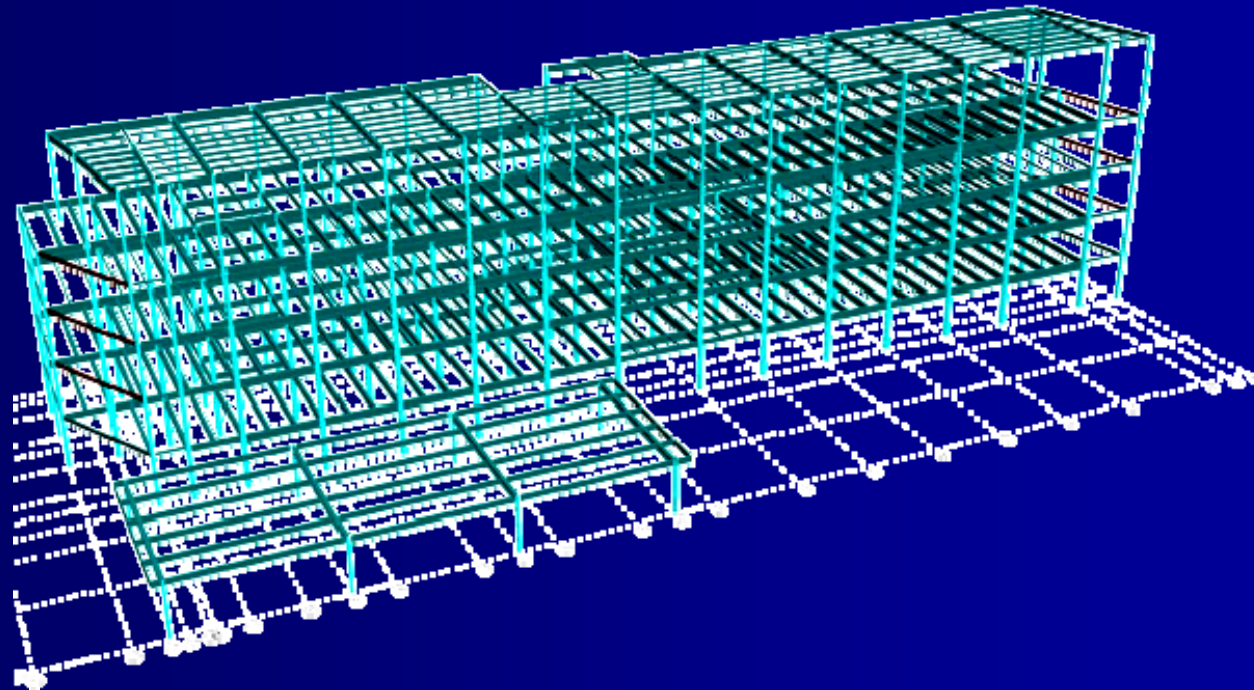
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Deflection Criteria:

Live: $1/360$, Total: $1/240$, & Vibration Criteria



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DEPTH STUDY

GRAVITY ANALYSIS

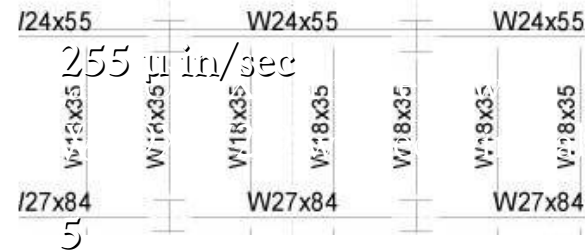
Design B

Vibrat

ing slowly:

5" slab over

Criteria



≤ 8,000

microscopes at up to 100x magnification

s, surgery, and bench



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Breadth Study

- Construction Management

- Architectural Analysis

Summary and Conclusions

Acknowledgements



FDA CDRH Laboratory

Timothy Mueller • Senior Thesis • Spring 2006 • Structural Option

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Deflection Criteria:

$$h/400$$

Seismic Deflection Criteria:

$$0.02h/\text{floor}$$

No damage to building systems ($h/180$)

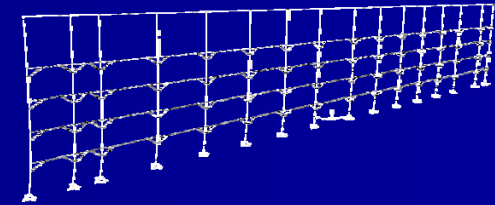
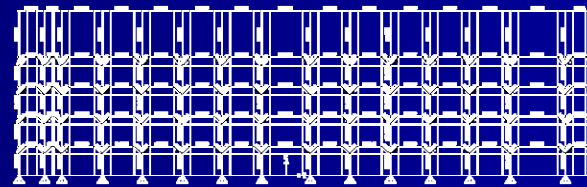
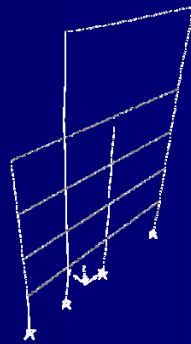
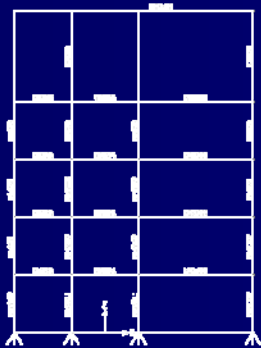


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DEPT STUDY
 •
 LATERAL ANALYSIS

Design A



Moment Frame Second Redesign		
Story	Allowable Drift (in.)	Story Drift (in.)
1	0.4830	0.4688
2	0.4830	0.2619
3	0.4830	0.2230
4	0.4830	0.1829
Penthouse	0.7599	0.3941
Full Building	3.1749	1.5307

Typical
Column:

W14

Moment
Frames:

3

Braced Frame First Redesign		
Story	Allowable Drift (in.)	Story Drift (in.)
1	0.4830	0.3367
2	0.4830	0.2280
3	0.4830	0.1947
4	0.4830	0.1748
Penthouse	0.7599	0.2740
Full Building	3.1749	1.1416

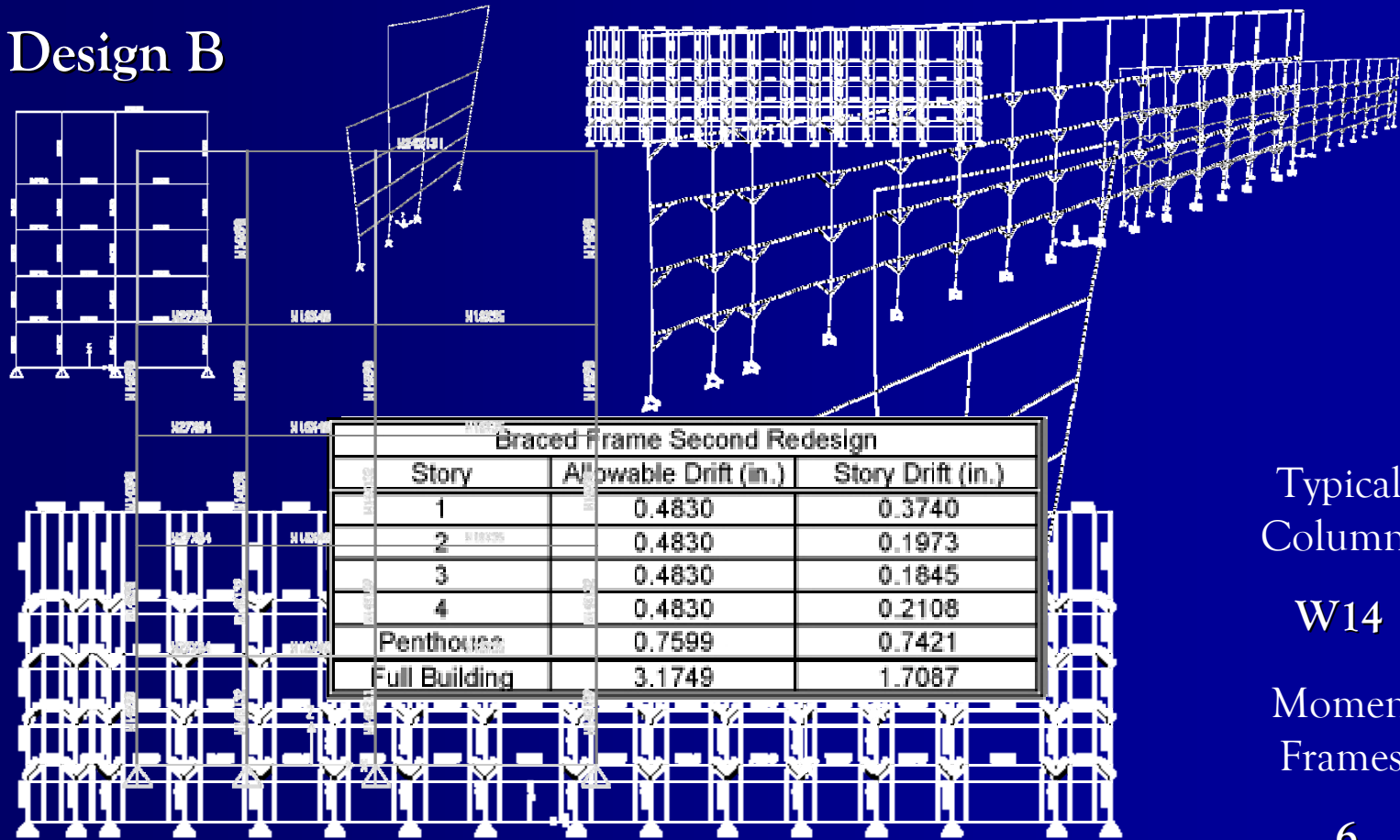


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LATERAL ANALYSIS

Design B



Typical Column:

W14

Moment Frames:



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Summary and Conclusions

Acknowledgements



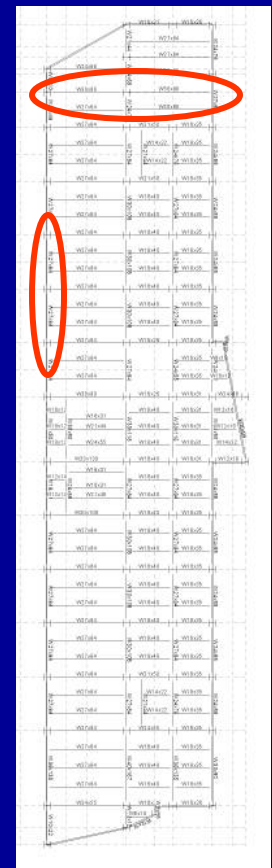
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ADDITIONAL CONDITIONS

Blast Control:

- **Location** - center of the limited access White Oak campus
- **One road access point** - north end of the building
- **No interior below grade parking garages**
- **Extra layer of welded wire mesh** in upper portion of the deck
- **Moment connections**
- **Square columns** - HSS shapes versus W-shape resistance torsion
- **progressive collapse beam**
support the load of two bay spans without deflection criteria
W40X230 to W40X431
- **Overall cost of a blast resistant system as compared to a non-resistive**
5% increase





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Height

Total height increase: 8.25'

- No height restrictions
- Slight increase in wind loads
- Minimal additional cladding cost

Weight/Foundation

Total mass decrease: $\frac{1}{4}$ original design (just under 6 million kips)

- Lower seismic forces
- Foundations reduced to $\frac{1}{3}$ original area

Fireproofing

Compatible spray-on fireproofing

- Decking: $\frac{3}{8}$ "
- Beams and girders: 1"
- Columns: $1\text{-}\frac{3}{8}$ "



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Considerations:

Concrete Pros:

- High Vibration Stability
- Integrated Fireproofing
- Small Floor Sandwich

Concrete Cons:

- Labor Intensive
- Large Total Mass
- Steel Roof System

Proposed Solution:

Proposed Solution:

- Design B Steel Structure
- Construct the FDA CDRH Laboratory with Steel
 - Fewer members
 - High vibration control
 - Blast control
 - More moment connections



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Project Background

General Architecture

Existing Structure

Depth Study

- Gravity Analysis

- Lateral Analysis

- Additional Considerations

Breadth Study

- Construction Management

- Architectural Analysis

Summary and Conclusions

Acknowledgements



FDA CDRH Laboratory

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Cost:

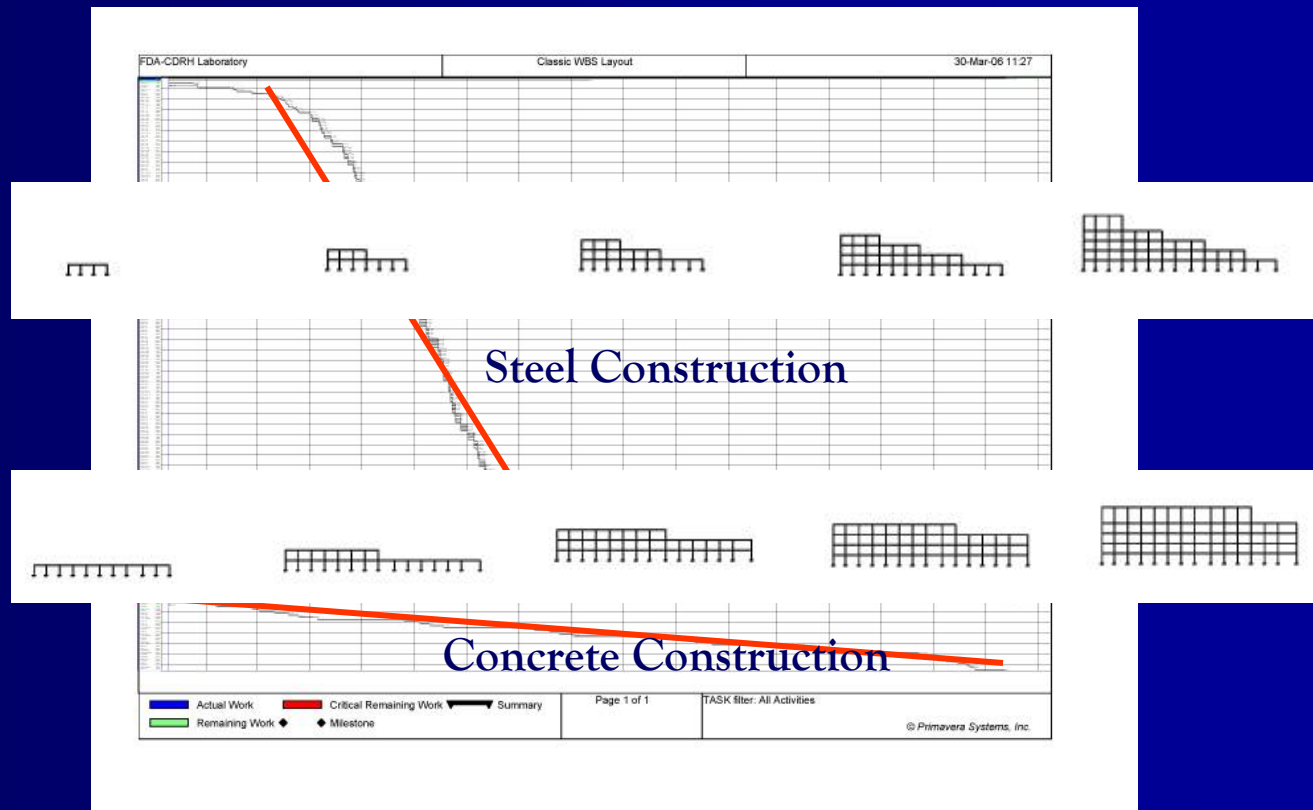
- | | |
|----------------------------------|---|
| • Current System (concrete) | \$4,492,275.00 |
| • Design A (steel spanning N-S) | \$6,929,354.00
\$6,929,354.00 |
| • Design B (steel spanning E-W) | \$1,100,052.00 |
| • Design B with Blast Resistance | \$9,306,443.85
\$9,306,443.85 |



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Considerations:

Concrete Pros:

- High Vibration Stability
- Integrated Fireproofing
- Small Floor Sandwich

Concrete Cons:

- Labor Intensive
- Large Total Mass
- Steel Roof System

Proposed Solution:

Proposed Solution:

- Design B Steel Structure
- Design B Steel Structure
 - Fewer members
 - Fewer members
 - Increased vibration control
 - High Vibration control
 - More moment connections
 - Blast control
 - Cost savings
 - More moment connections
 - Time savings

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Project Background

General Architecture

Existing Structure

Depth Study

- Gravity Analysis

- Lateral Analysis

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Breadth Study

- Construction Management

- Architectural Analysis

Summary and Conclusions

Acknowledgements



FDA CDRH Laboratory

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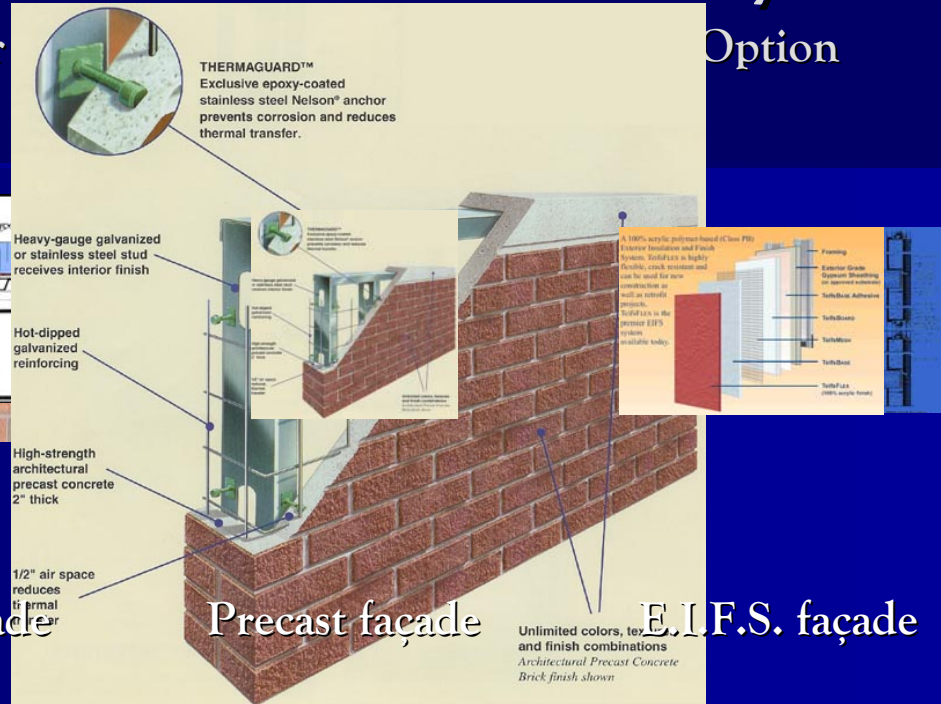
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Steel façade



Brick façade



Precast façade

EIFS façade

W27X84 to W30X90
W21X50 to W24X76
W18X40 to W21X48



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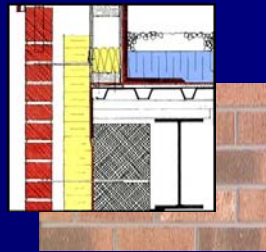
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Steel façade

\$1,086,093.35



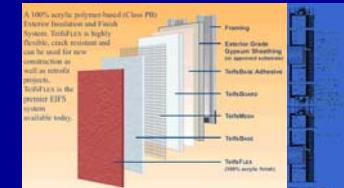
Brick façade

\$1,592,509.07



Precast façade

\$1,457,993.45



E.I.F.S. façade

\$1,037,865.72



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Concrete Pros:

- High Vibration Stability
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- Small Floor Sandwich

Concrete Cons:

- Labor Intensive
- Large Total Mass
- Steel Roof System

Proposed Solution:

Proposed Solution Structure

- Design B Steel Structure
 - Fewer members
 - Increased vibration control
 - Fewer members
 - More moment connections
 - Increased vibration control
 - Cost savings
 - More moment connections
 - Time savings
- Pre-Cast Facade
 - Cost savings
 - Fast installation
 - Time savings
 - Traditional image
 - Additional blast resistance



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- Architectural Analysis

Summary and Conclusions

Acknowledgements



FDA CDRH Laboratory

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Current Building
(concrete structure & steel façade) **\$5,578,368.35**

Proposed Building
(steel structure & steel façade) **\$4,364,492.50**

Proposed Building
(steel structure & precast façade) **\$5,413,682.56**



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Current Building \$5,578,368.35
(concrete structure & steel façade)

Time Savings

Greater Than Satisfactory Vibration Control
Proposed Building \$930,440.85
(steel structure & steel façade)

Equivalent Fireproofing

Proposed Building \$441,540.75
Campus Unifying Façade
(steel structure & precast façade)

Smaller Foundation

Increased Blast Protection



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Breadth Study

- Construction Management

- Architectural Analysis

Summary and Conclusions

Acknowledgements



FDA CDRH Laboratory

Timothy Mueller • Senior Thesis • Spring 2006 • Structural Option

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I would like to thank the following people:

James Piedrafita, Truland Systems Corporation, for providing me with all of my resources, as well as a work experience and knowledge that can not be quantified.

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My Friends, who without their help, support, and ear, I would never have been able to survive this past year.

and

My Family, who not only provided me with a sounding board this past year, but a sound foundation to build my future from.



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Questions?