

# The University Sciences Building

Northeast, USA



Final Presentation - 4.9.2012

Chris Dunlay

Structural Option

Dr. Boothby

## Project Facts

- Located in Northeast, USA
- 209,000 S.F.
- 2 below grade stories / 7 above grade stories
- Maximum Height: 117'-0"
- Classroom, Laboratories, and Offices
- Construction Cost: \$ 80 Million
- August 2007 – December 2009
- LEED GOLD Certification





## Thesis Topics

- Existing Structural System
- Problem and Solution
- Gravity Design
- Lateral Design
- Construction Management Study
- Mechanical System Study





## Project Team

- **Owner:** Not Release
- **Architect:** Mack Scogin Merrill and Elam
- **General Contractor:** PJ Dick
  - **WEB Contractor:** Graziano Construction
- **Structural Engineer:** ARUP – Boston
- **Mechanical Engineer:** ARUP – Boston
- **Electrical Engineer:** ARUP - Boston
- **Civil Engineer:** Civil and Environmental Consultants



**Mack Scogin Merrill Elam Architects**





## Architecture

- Independent from surrounding campus architecture
- Unique Façade:
  - Zinc Panels
  - Aluminum Window Trim
- Multiple Atriums
- Unsymmetrical floor plans





## Existing Structural System

- **Superstructure (4 – Roof)**
  - Composite Deck on steel framing
- **Foundation (Levels 1-3)**
  - Drilled caissons, strip and column footings
  - Concrete walls and columns
  - 150 car parking garage
- **Lateral System**
  - Dual Shear Walls and Braced Frames





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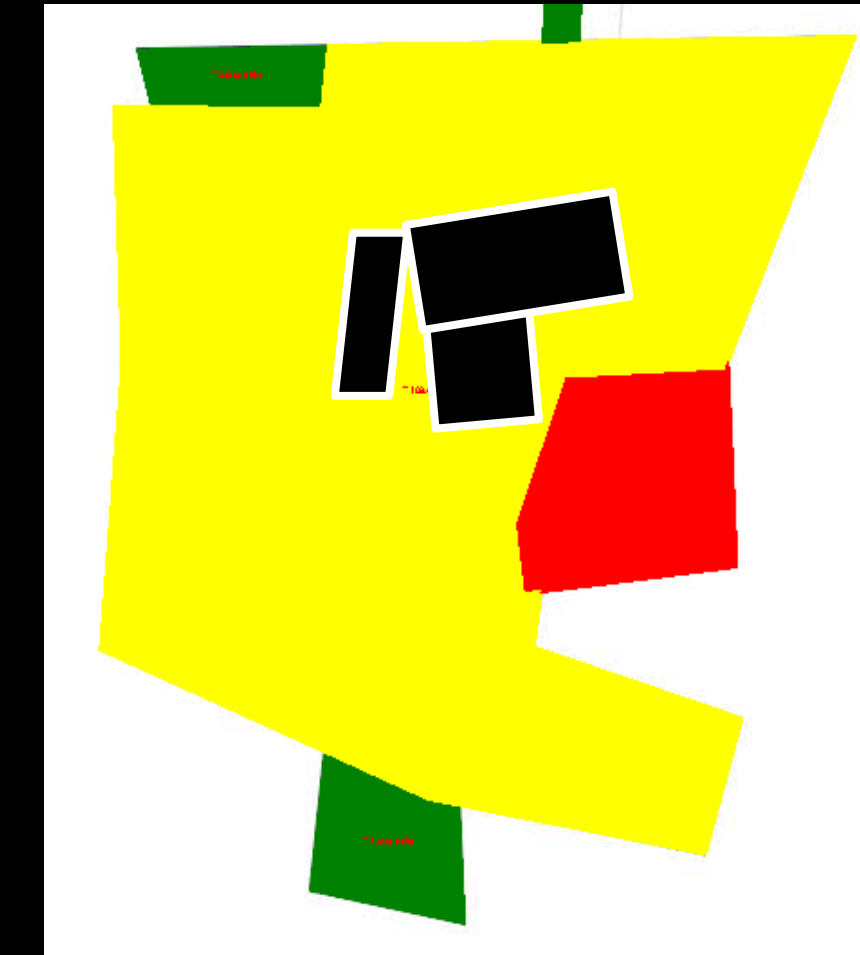


Braced Frame

Shear Wall

## Roof Elevations

- **6<sup>th</sup> Level Roof – 61'-0"**
- **7<sup>th</sup> Level Roof – 75'-0"**
- **9<sup>th</sup> Level Roof – 103'-0"**
- **Mechanical Penthouse – 117'-0"**





## Problem Statement

- **Superstructure Schedule**
  - Erection and detailing of steel put project 2 months behind schedule
- **Incurring Costs**
  - Delayed schedule added general condition costs
  - Change orders were frequent
  - Other trades inherently feel behind schedule





## Problem Solution

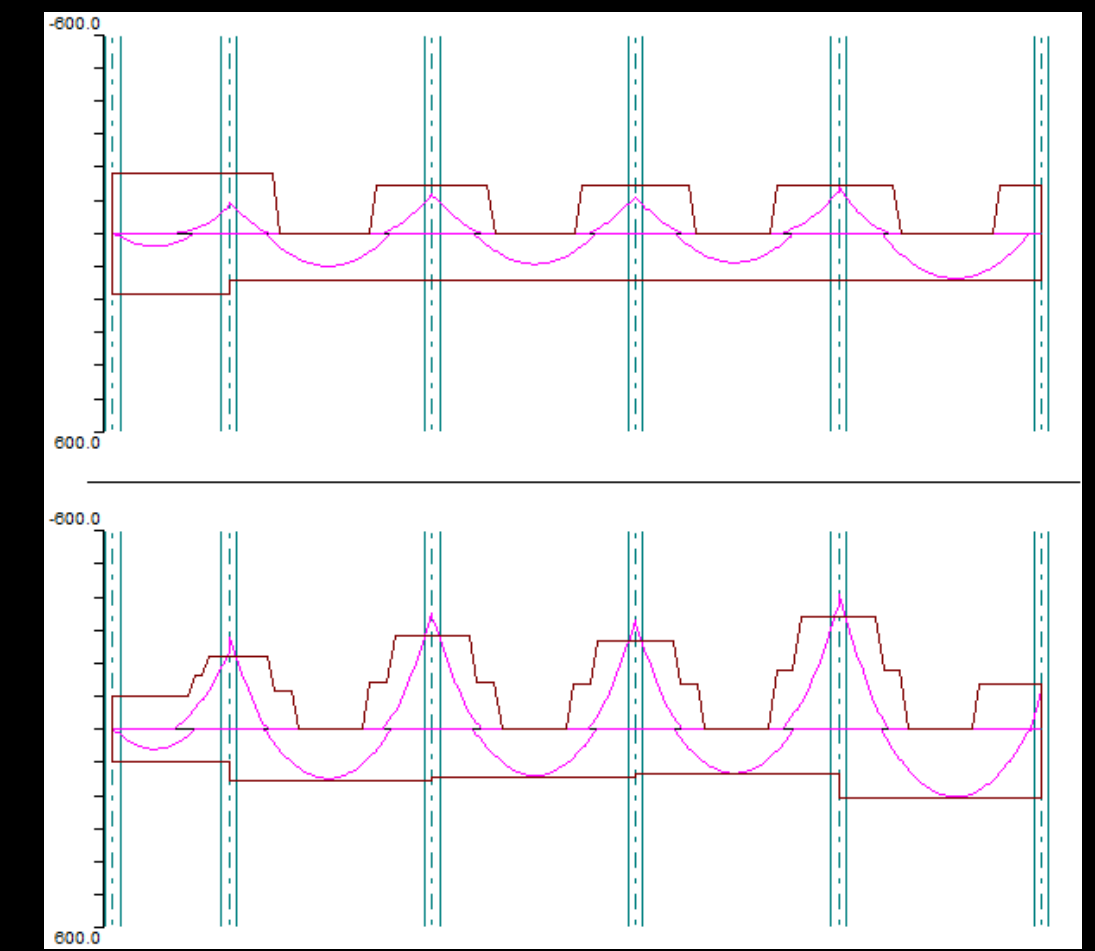
- **Concrete Structure**
  - One trade
  - No 'Connections'
  - Predictable and efficient schedule
- **Structural Systems**
  - Two Way Flat Plate
  - Shear Wall-Moment Frame Interactive System

## Goals

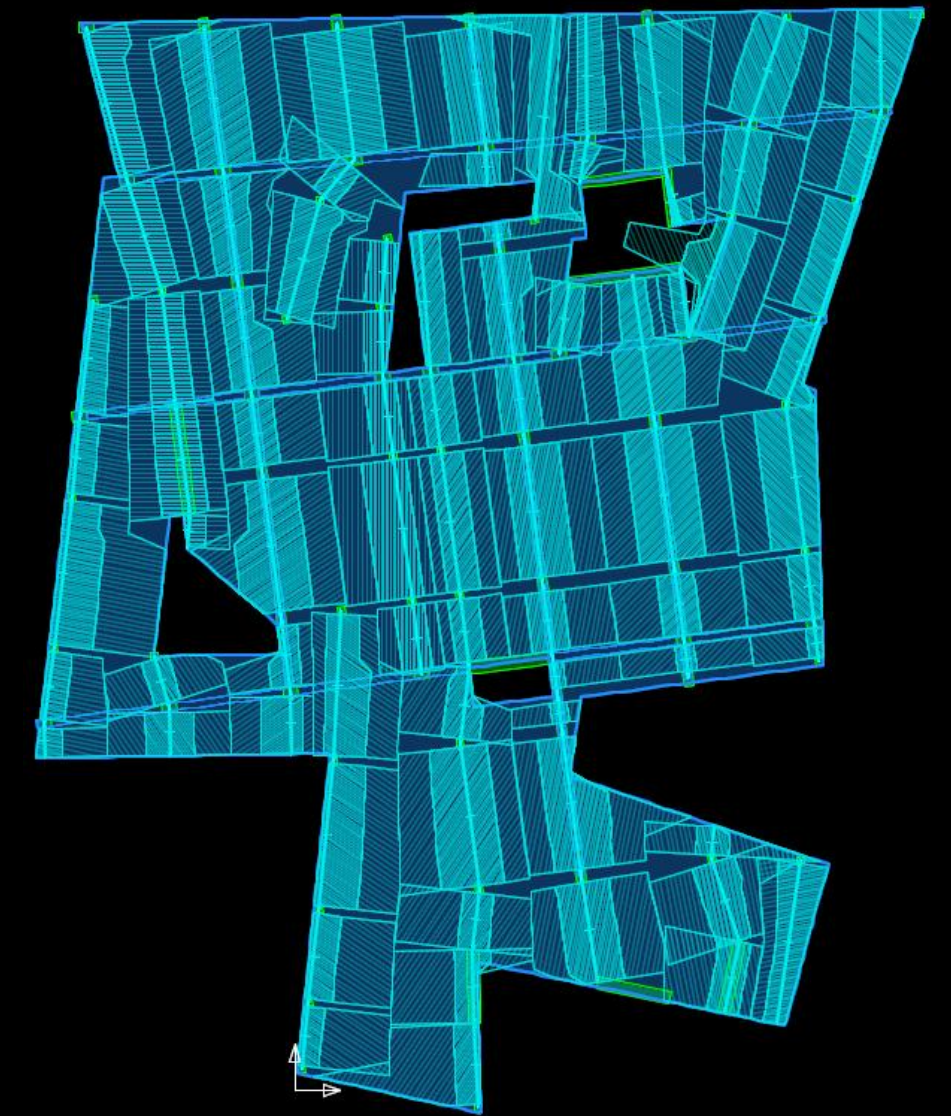
- **Concrete Design**
  - Manageable budget and schedule
  - Shear wall core
  - Moment frames to help reduce torsion
- **Truss**
  - Design truss to resist gravity loads on west cantilever



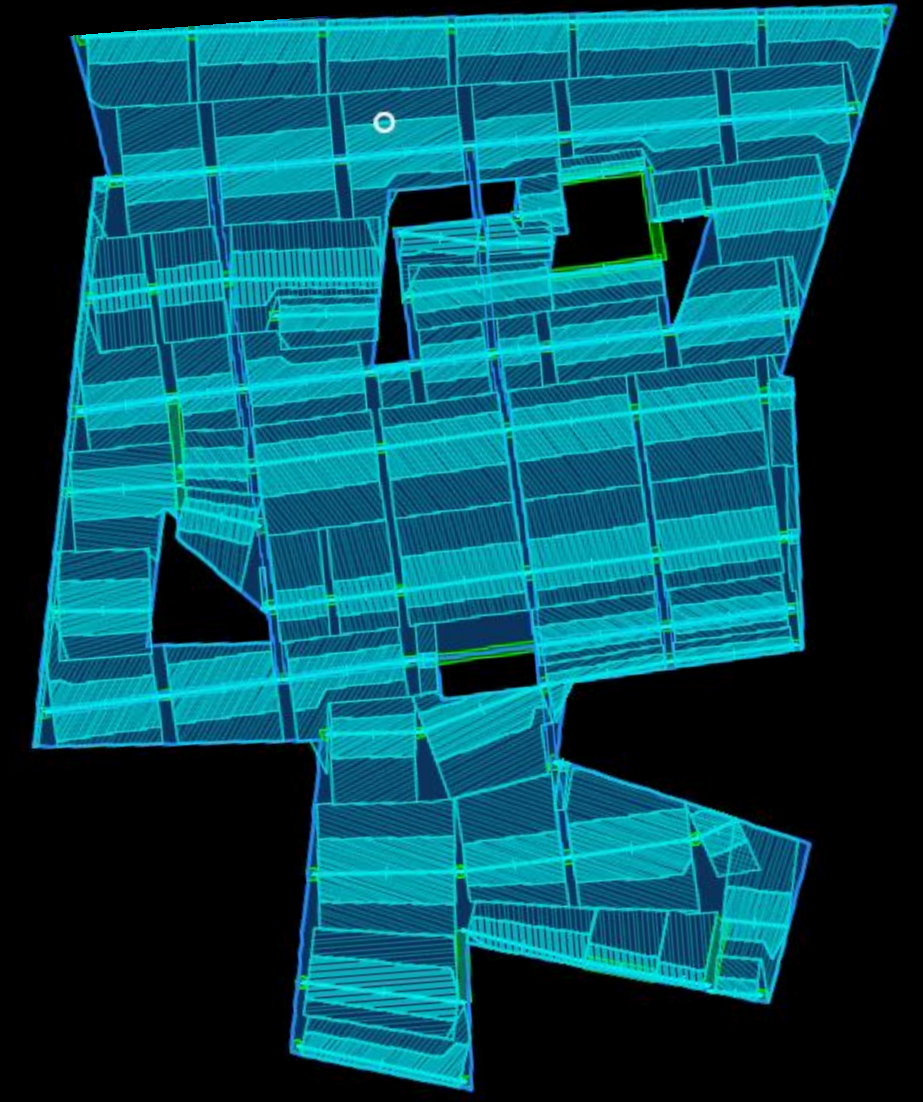
### Two Way Flat Plate Design



**spSlab:** Level 6 Equivalent Frame Analysis



**RAM Concept:** Level 6 Longitudinal Design Strips

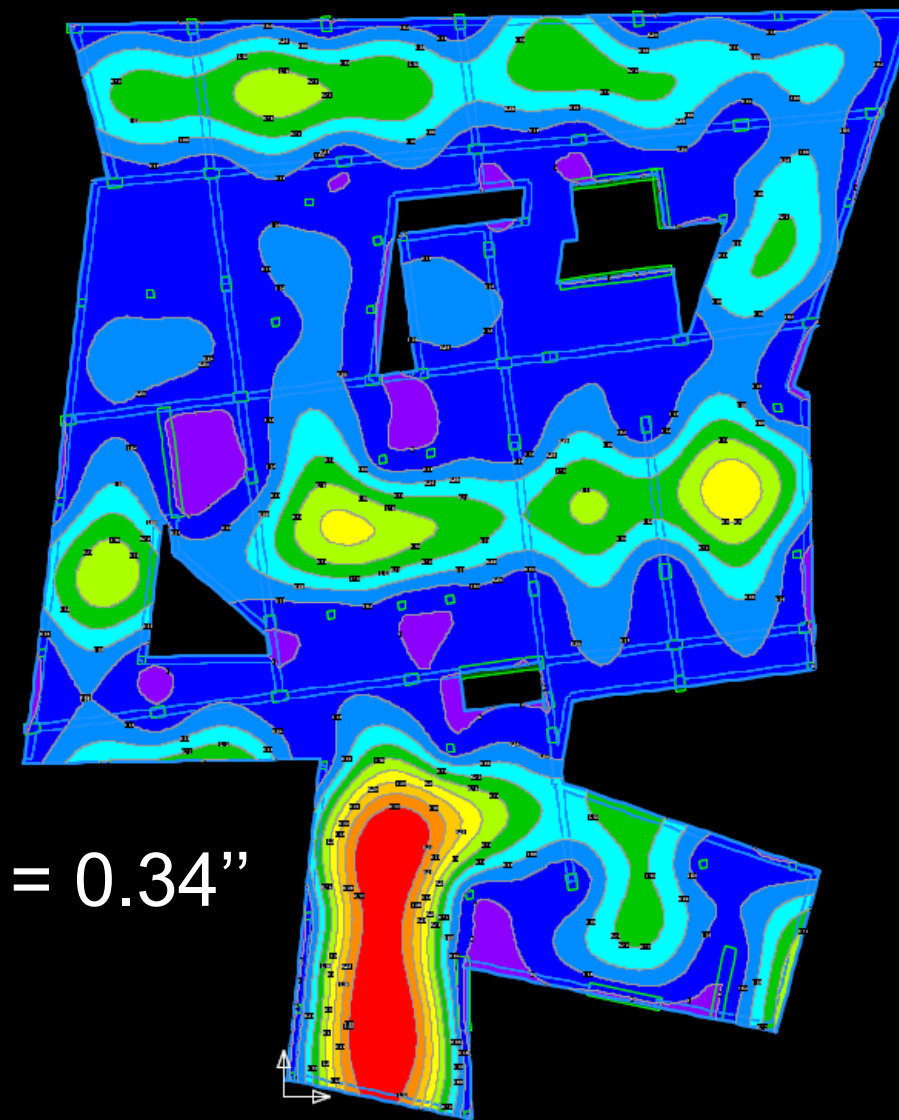


**RAM Concept:** Level 6 Latitude Design Strips

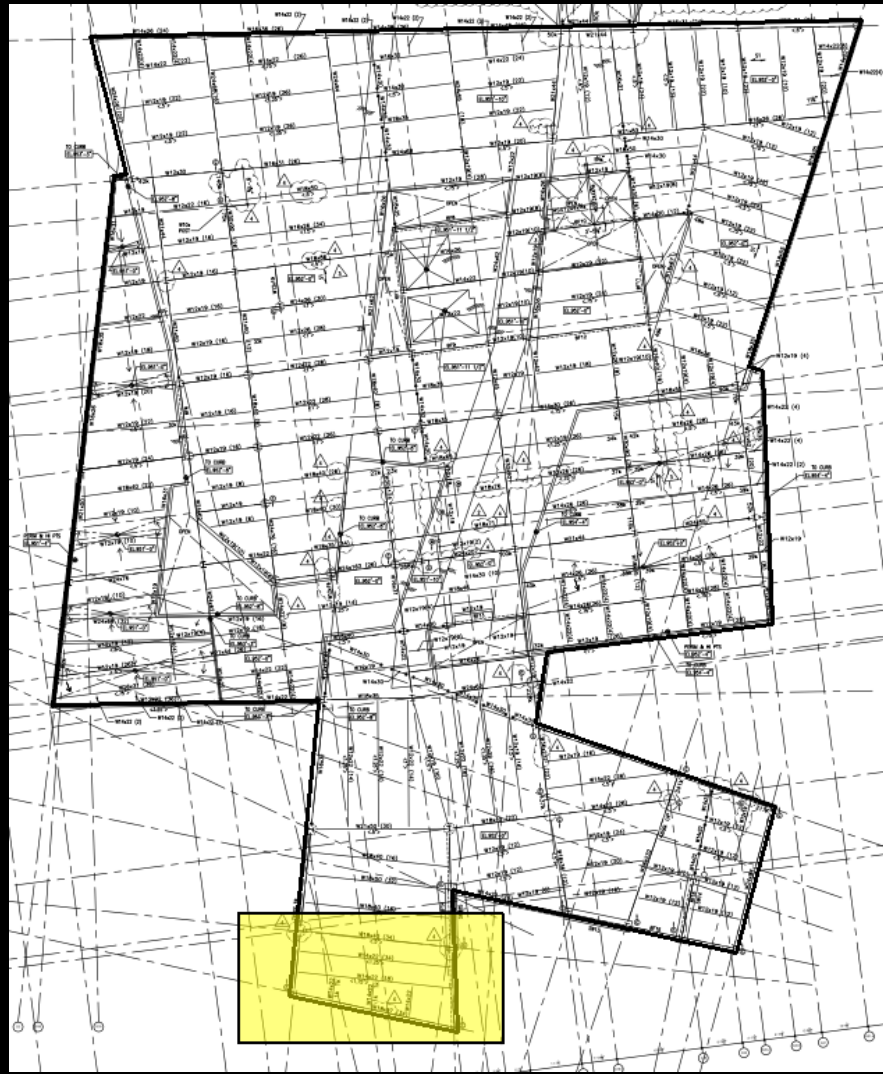
## Two Way Flat Plate Design

STRIP	LOCATION	MINIMUM PERCENT $A_s$ AT SECTION	WITHOUT DROP PANELS		WITH DROP PANELS	
			Diagram 1	Diagram 2	Diagram 3	Diagram 4
COLUMN STRIP	TOP	50 Remainder	$0.30 \ell_n$ $0.20 \ell_n$	$0.30 \ell_n$ $0.20 \ell_n$	$0.33 \ell_n$ $0.20 \ell_n$	$0.33 \ell_n$ $0.20 \ell_n$
	BOTTOM	100	6" At least two bars or wires shall conform to 13.3.8.5	Class B splices shall be permitted in this region	6" Continuous bars	6"
MIDDLE STRIP	TOP	100	$0.22 \ell_n$	$0.22 \ell_n$	$0.22 \ell_n$	$0.22 \ell_n$
	BOTTOM	50 Remainder	6"	Max. $0.15 \ell$ 6"	Max. $0.15 \ell$ 6"	6"

$$\Delta_{\max} = 0.34''$$

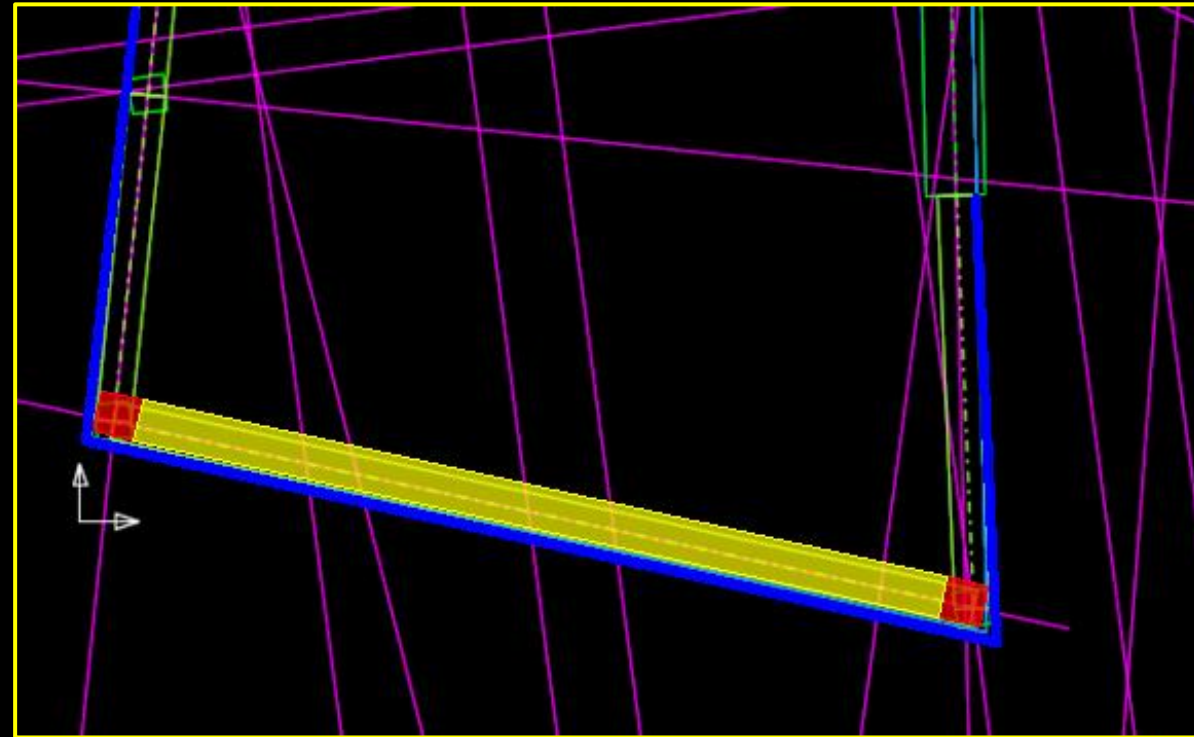






**Beam Edge Location**

### Edge Beam Design

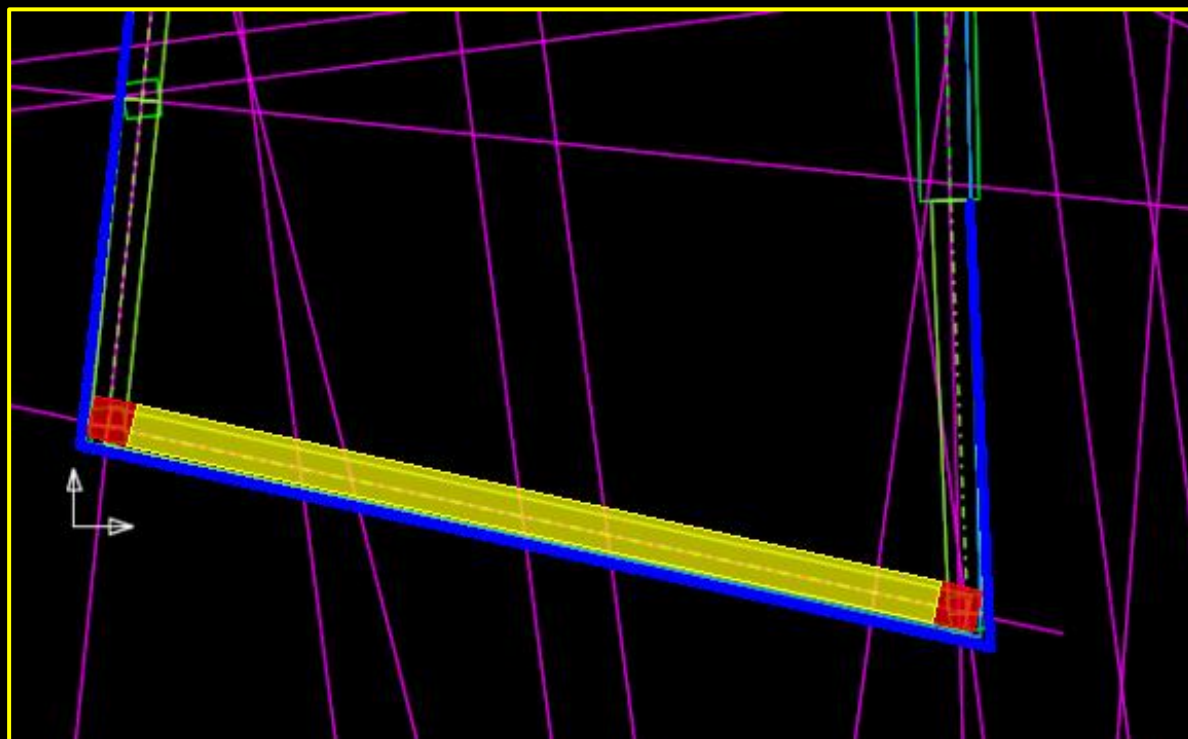


**Design Edge Beam Span**

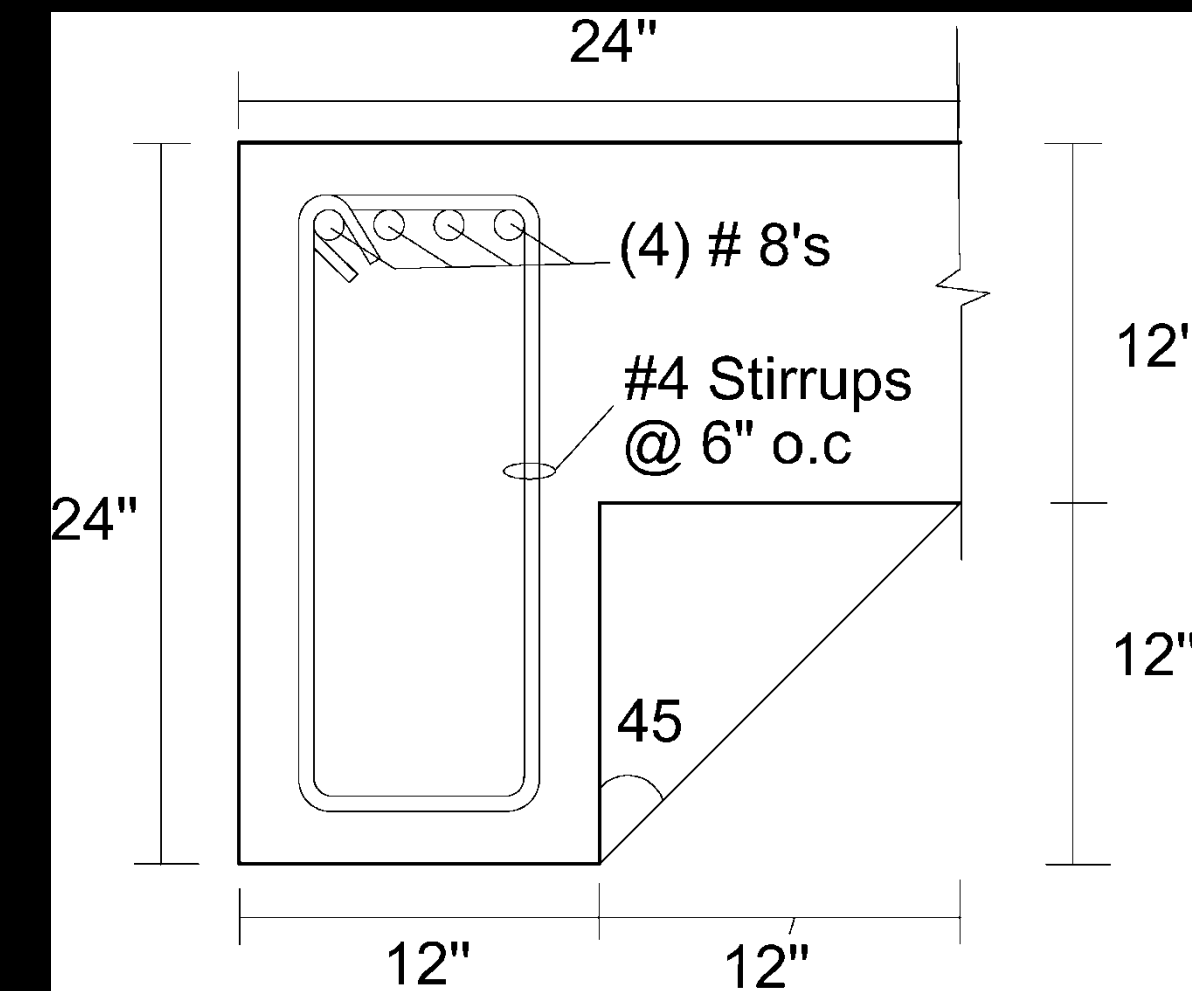
### Edge Beam Design



Beam Edge Location



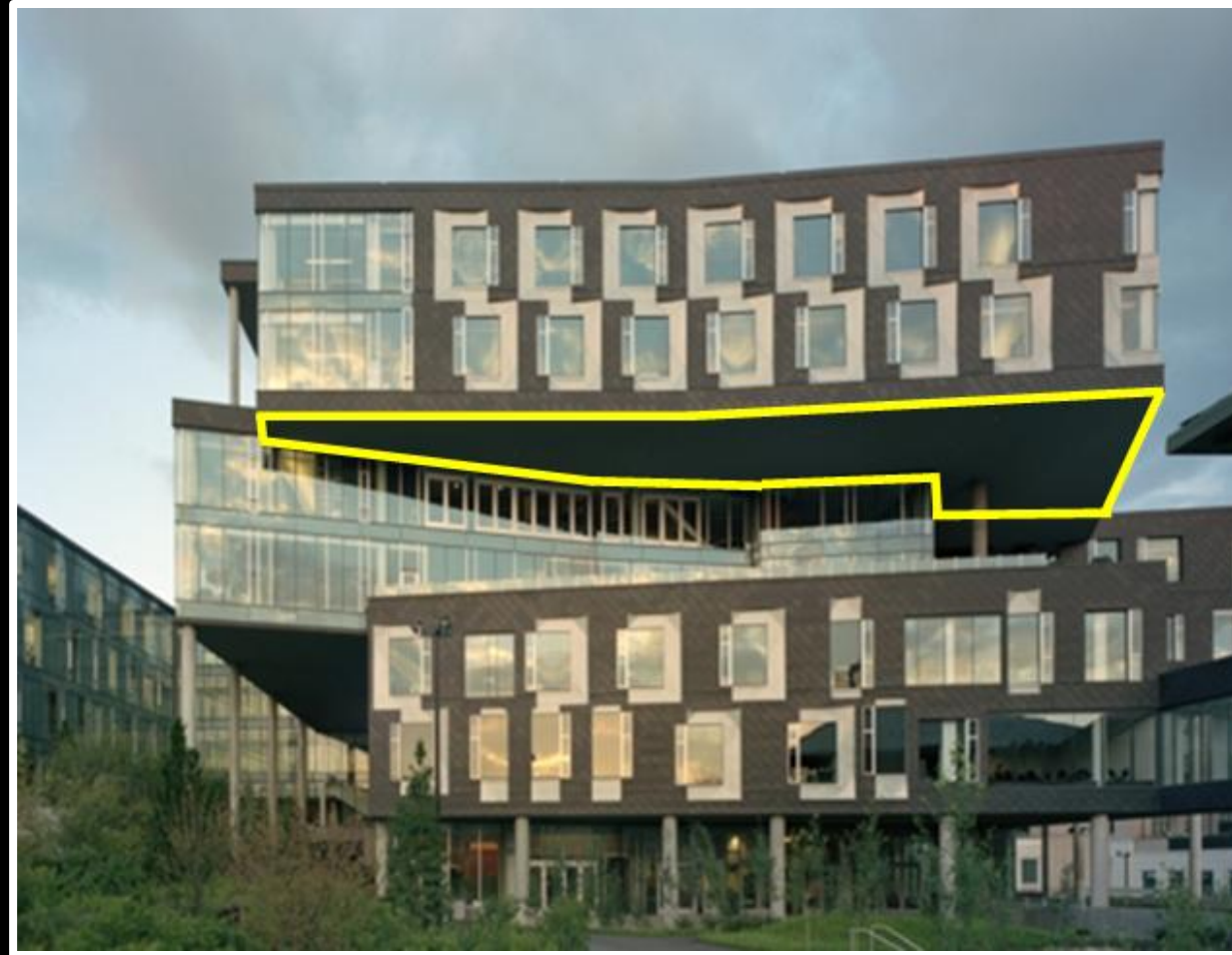
Design Edge Beam Span



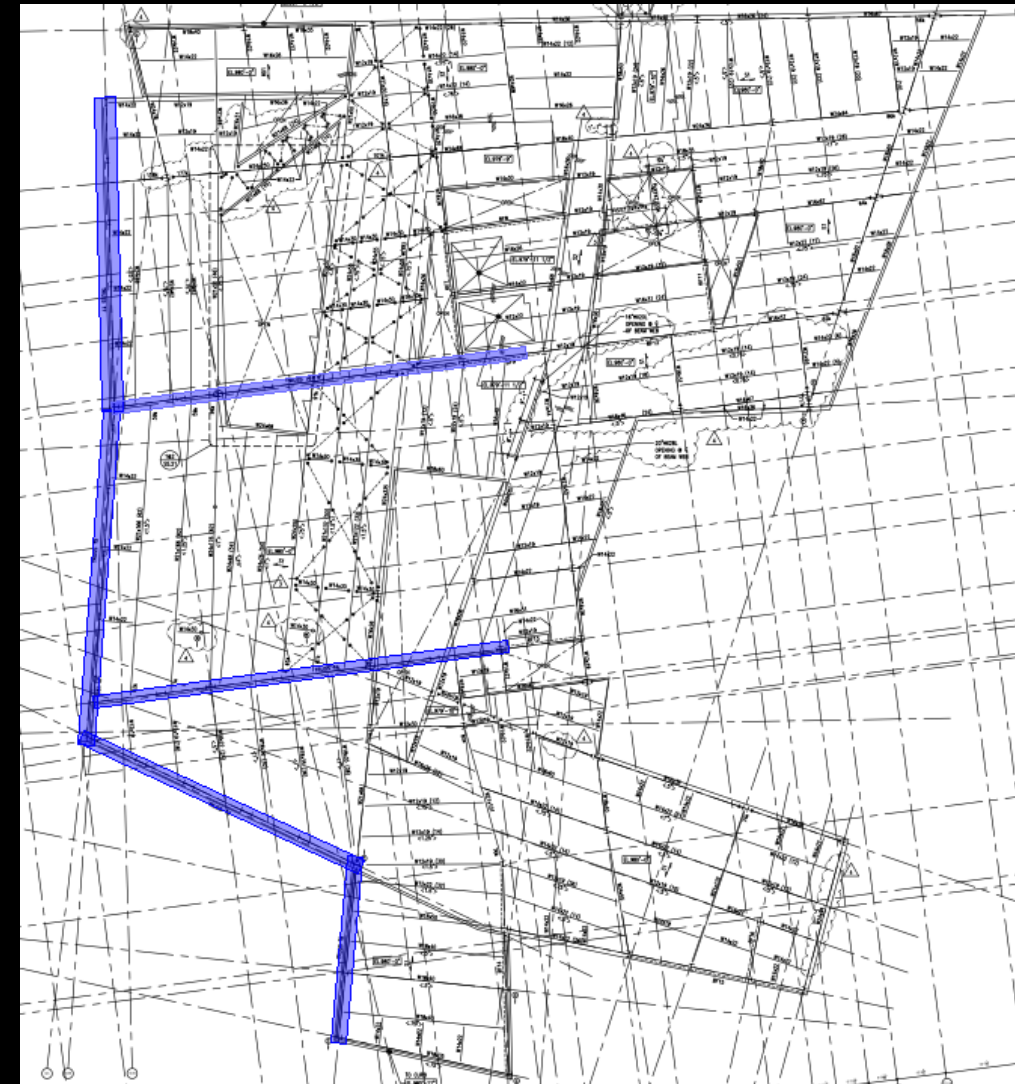
Designed Edge Beam Section



# Truss Design



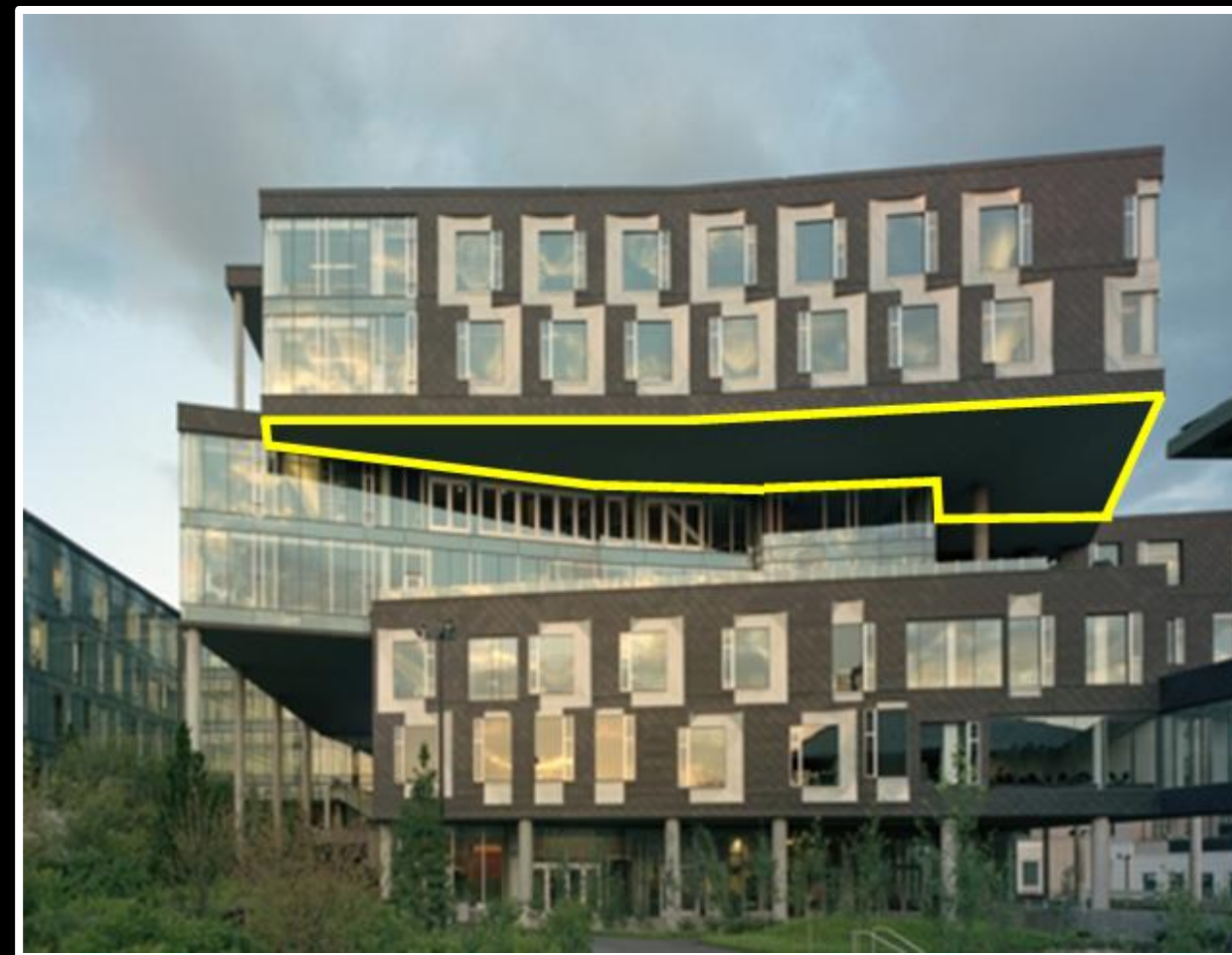
West Elevation: Cantilever Highlight



Level 6 Truss Plan



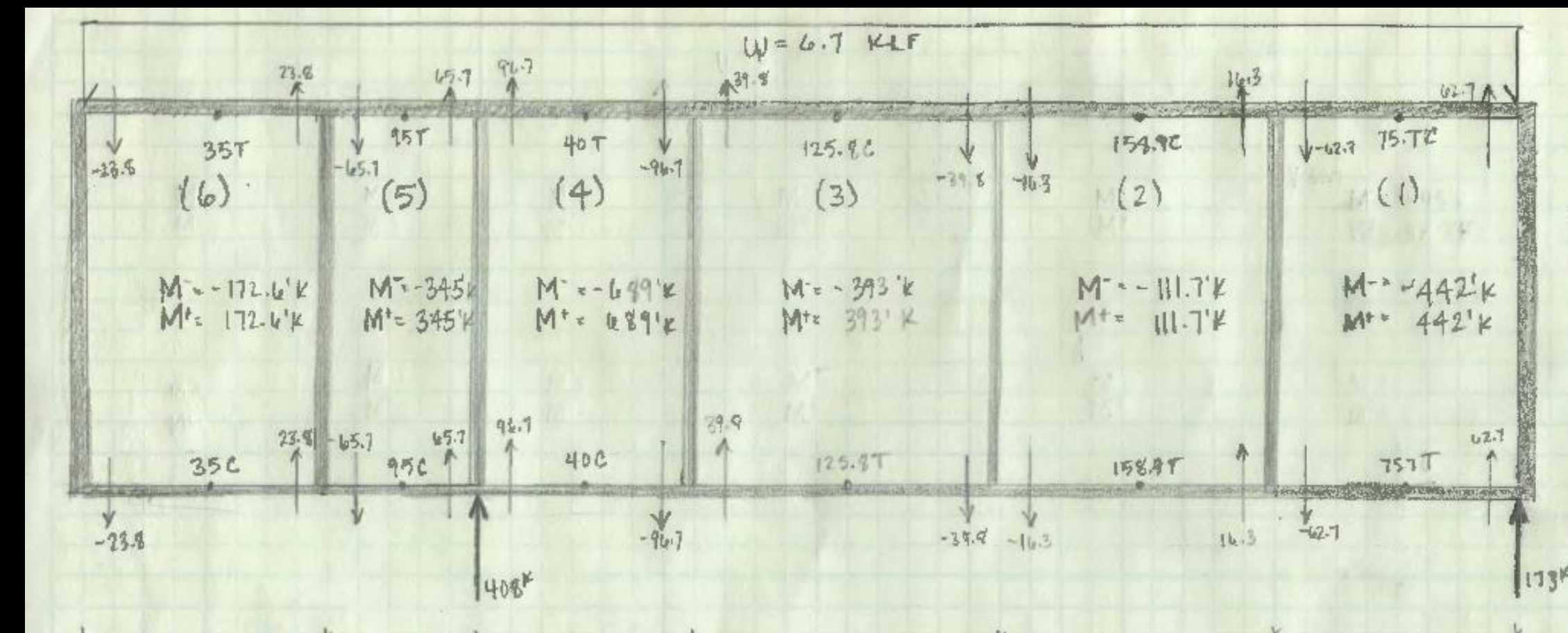
## Truss Design



West Elevation: Cantilever Highlight

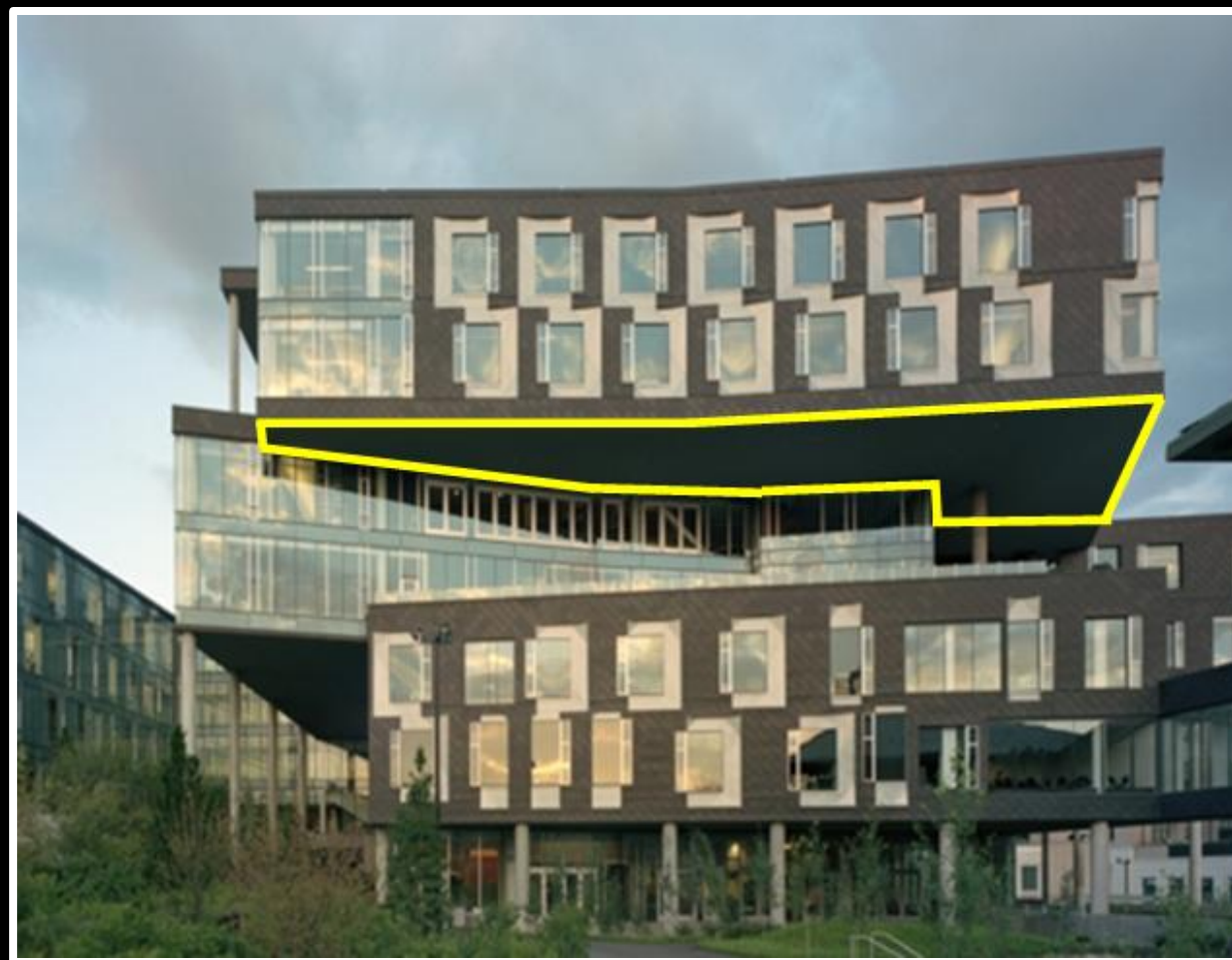
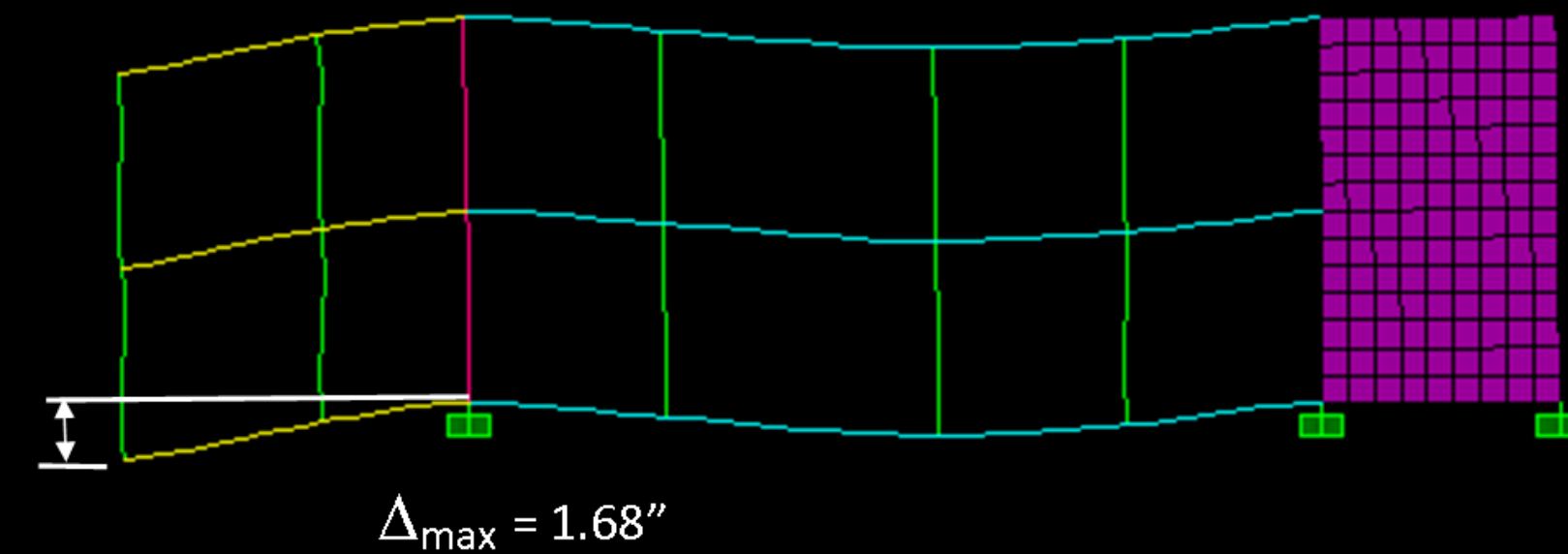


Level 6 Truss Plan – Frame GO Highlight



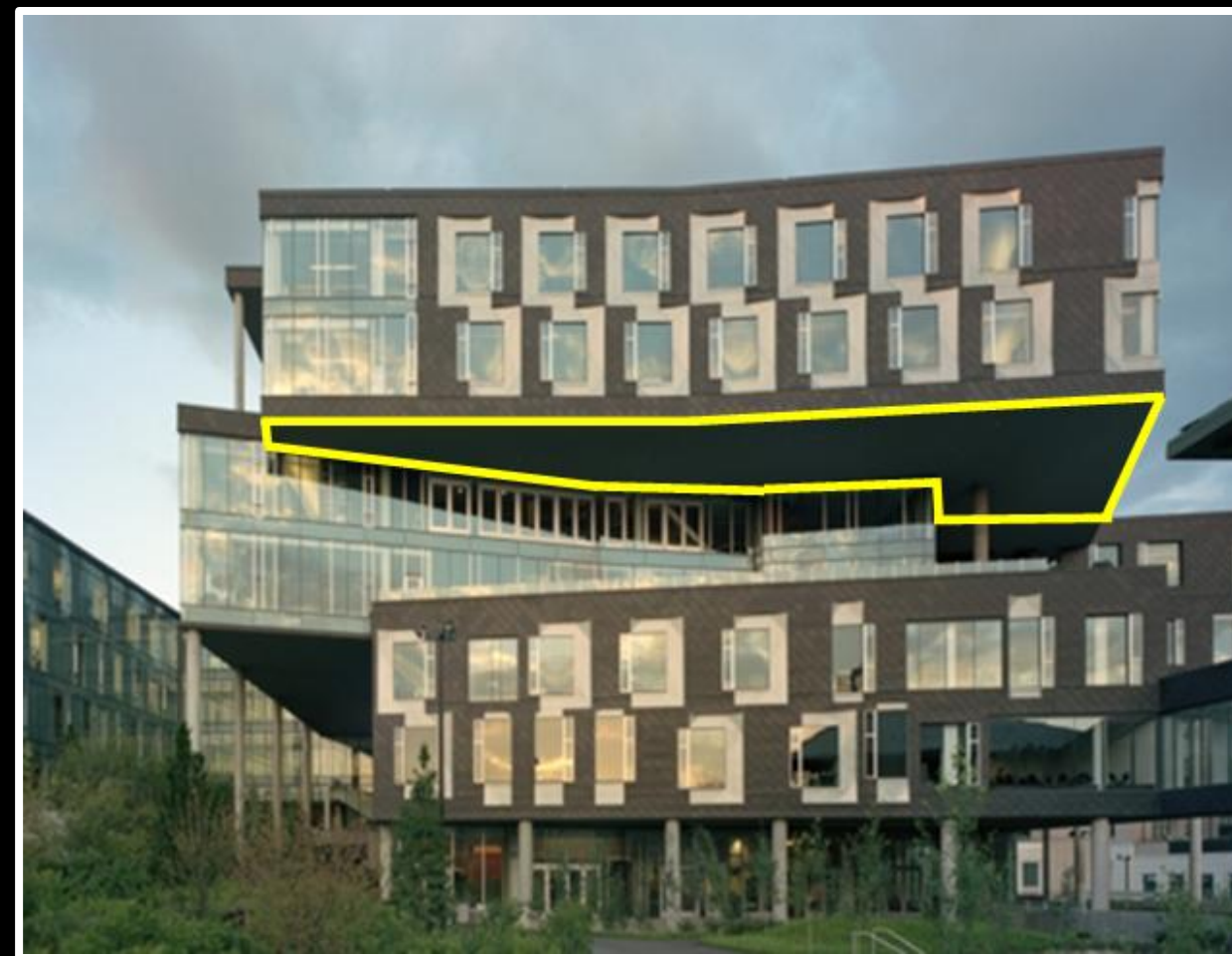
Frame GO: Portal Frame Analysis



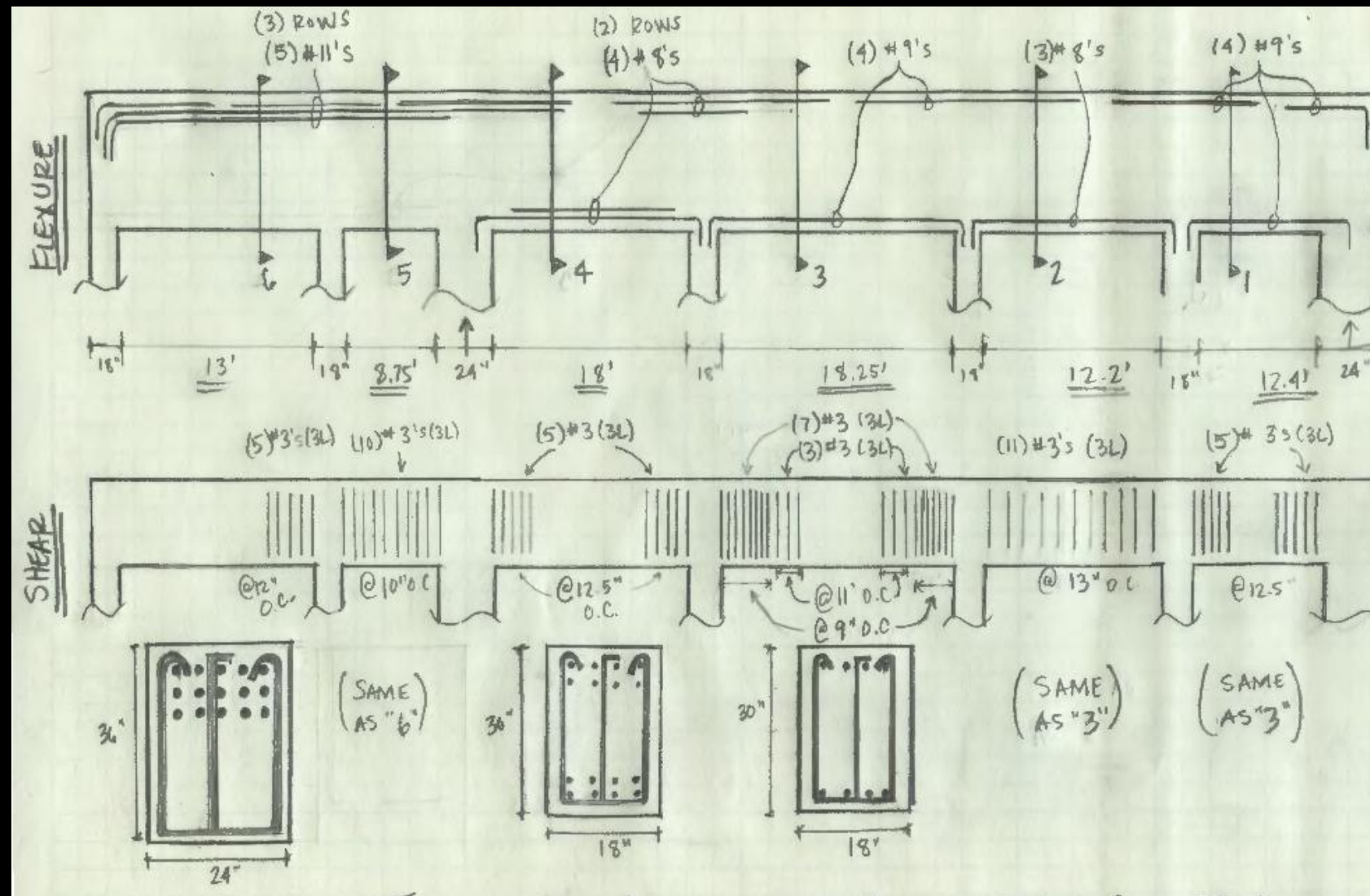
**Truss Design****West Elevation: Cantilever Highlight****Level 6 Truss Plan – Frame GO Highlight****Frame GO: ETABS Analysis**



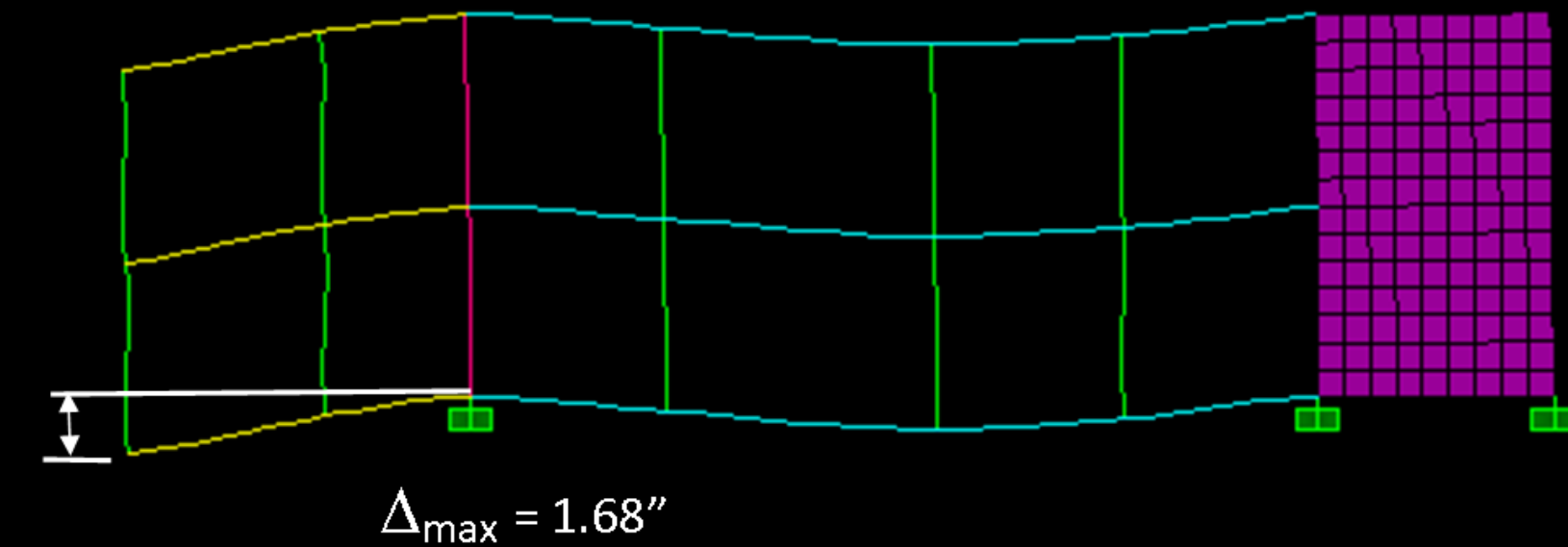
# Truss Design



West Elevation: Cantilever Highlight



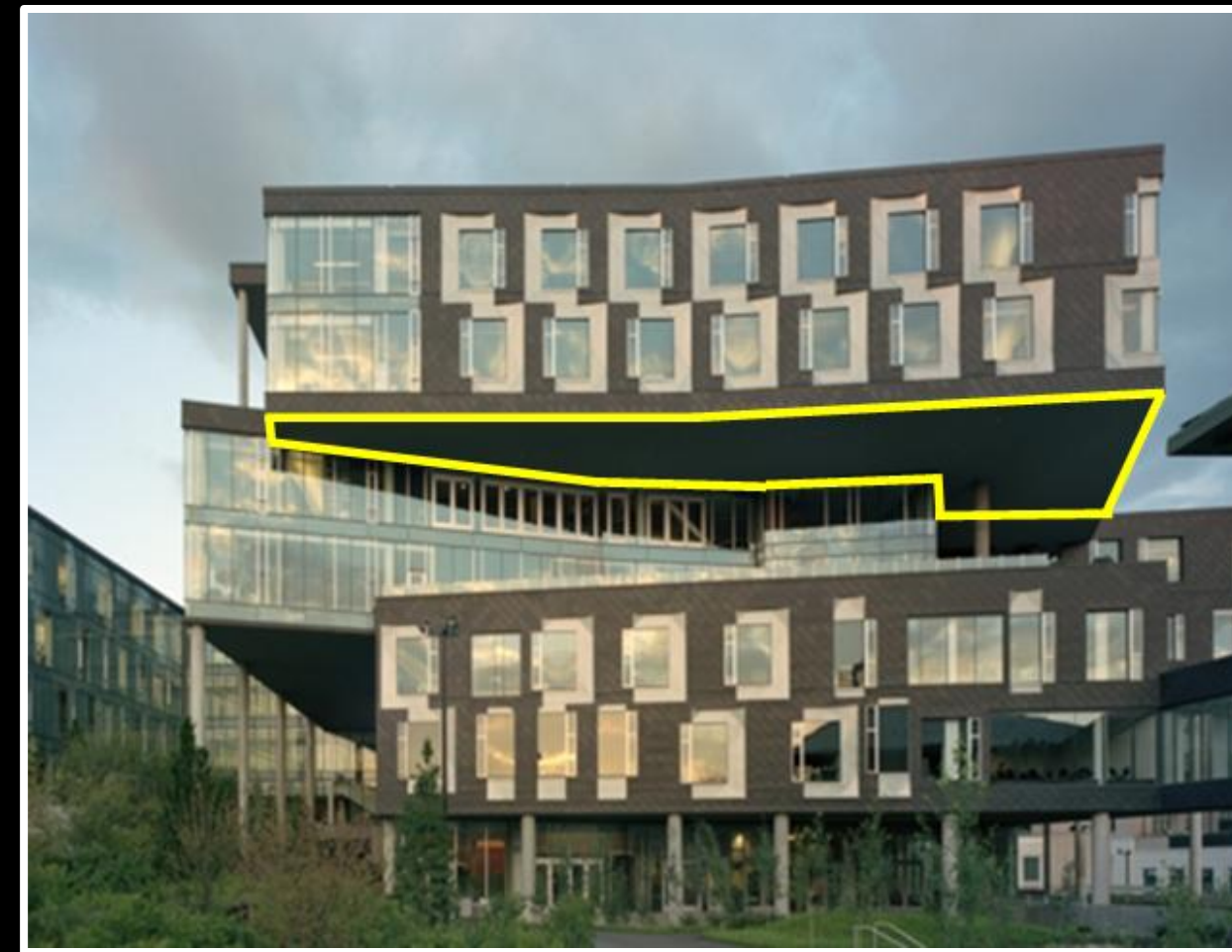
ETABS: 3D Truss



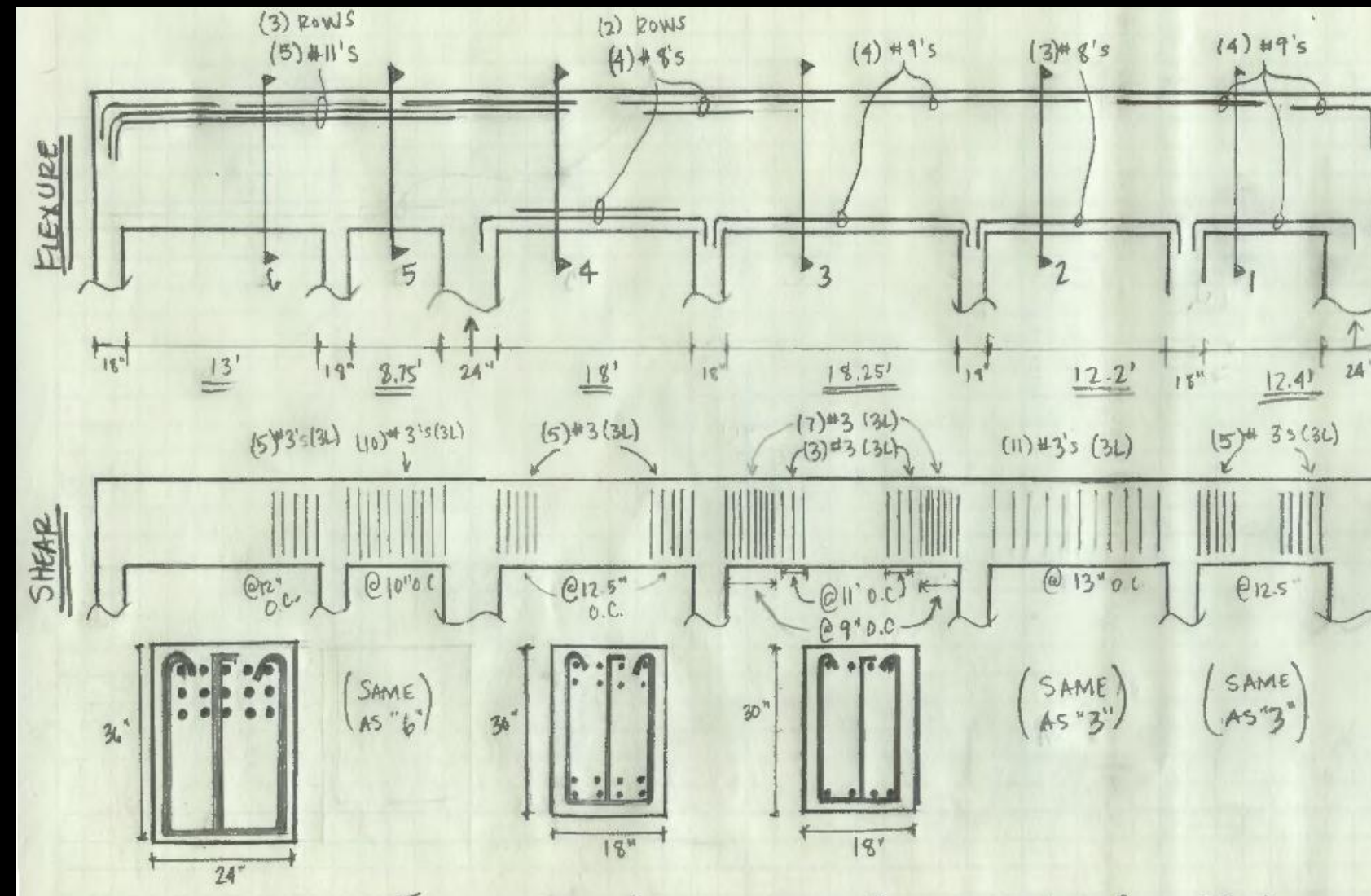
Frame GO: ETABS Analysis



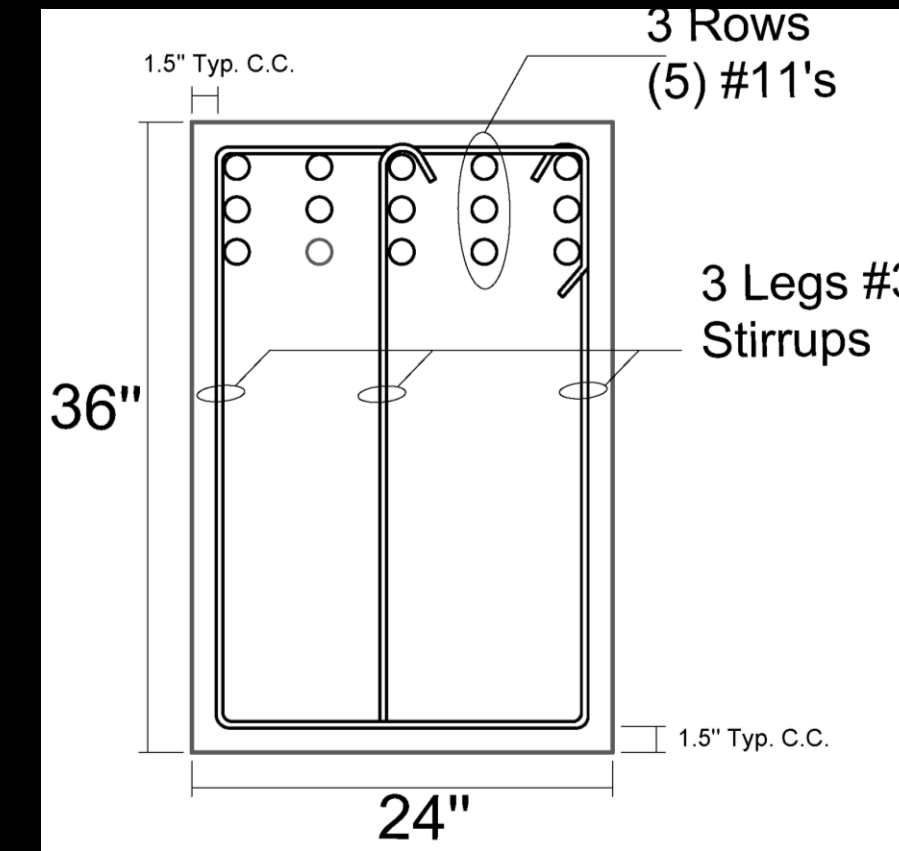
# Truss Design



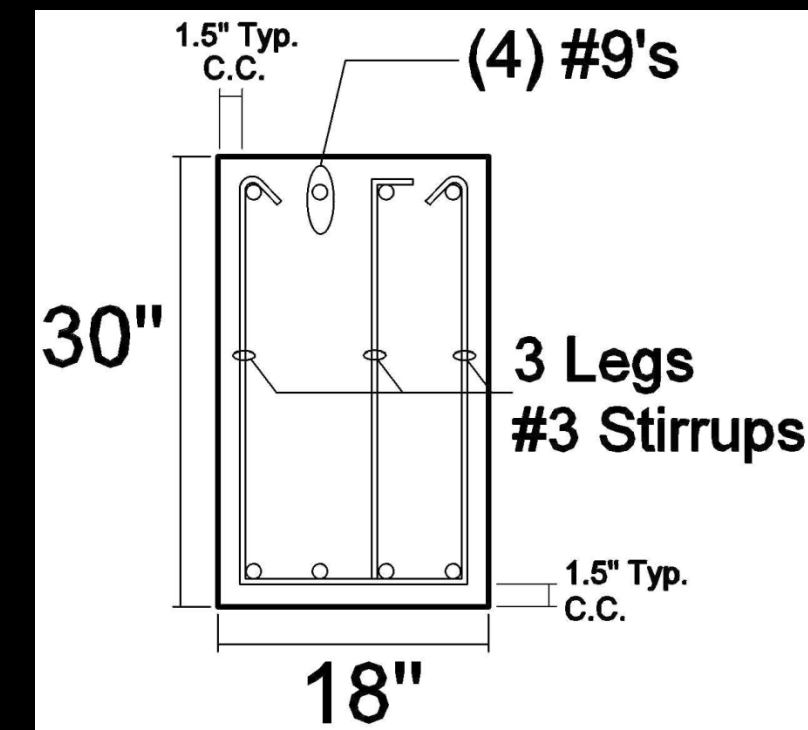
West Elevation: Cantilever Highlight



ETABS: 3D Truss



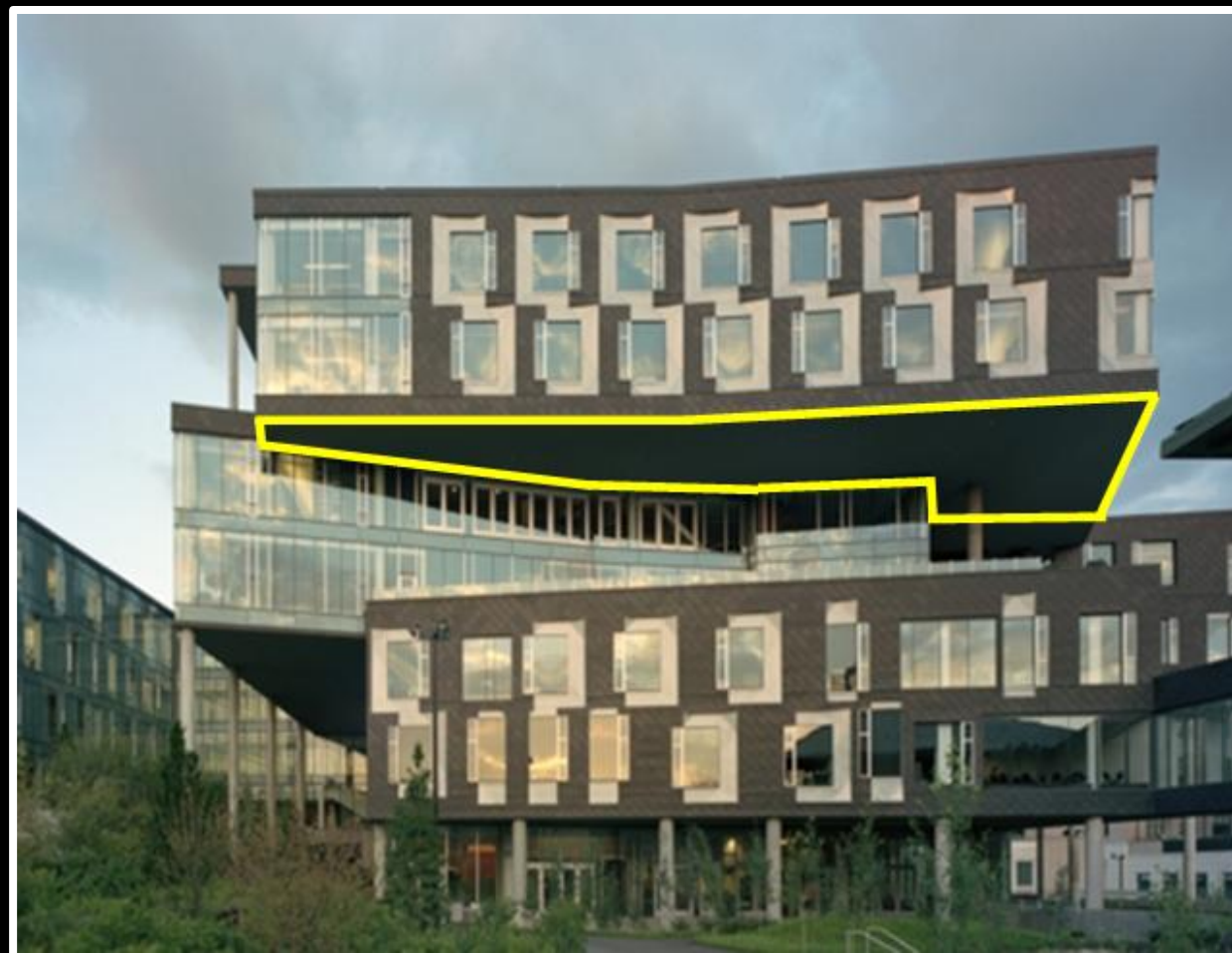
Cantilever Design Section



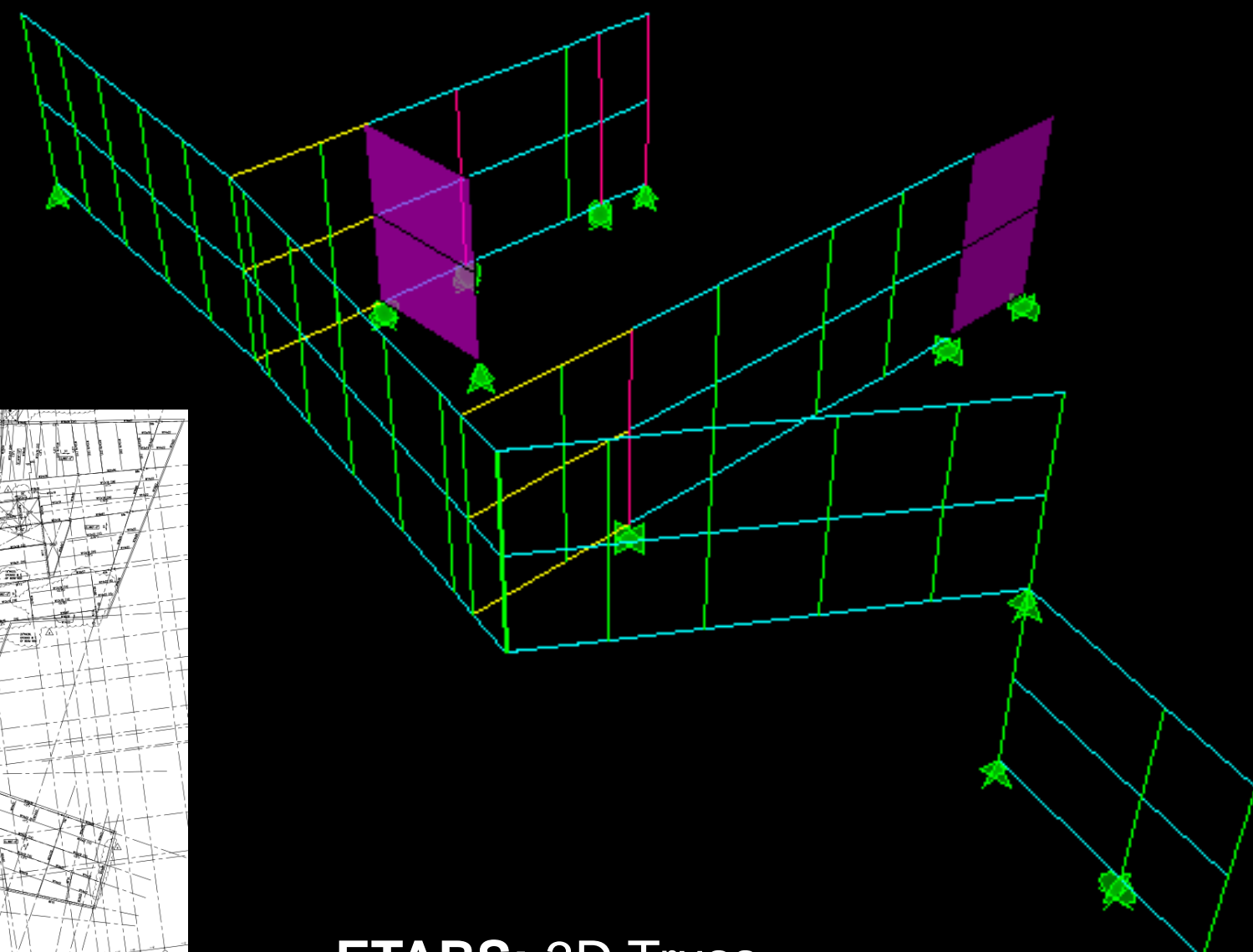
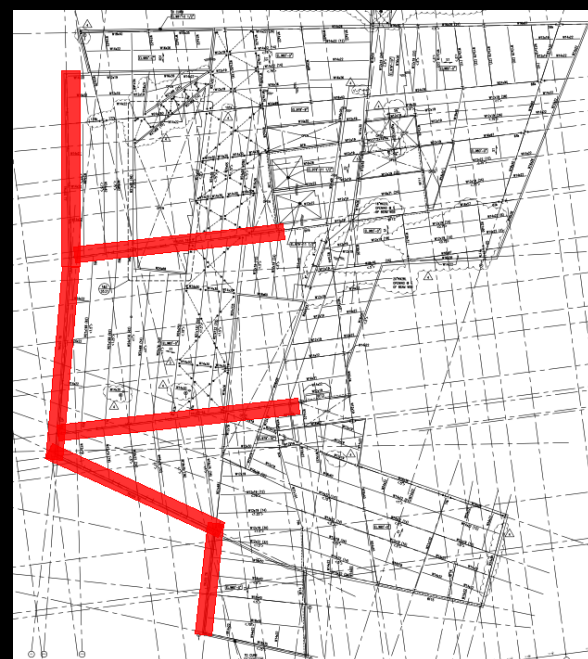
Mid-span Design Section



# Truss Design



West Elevation: Cantilever Highlight



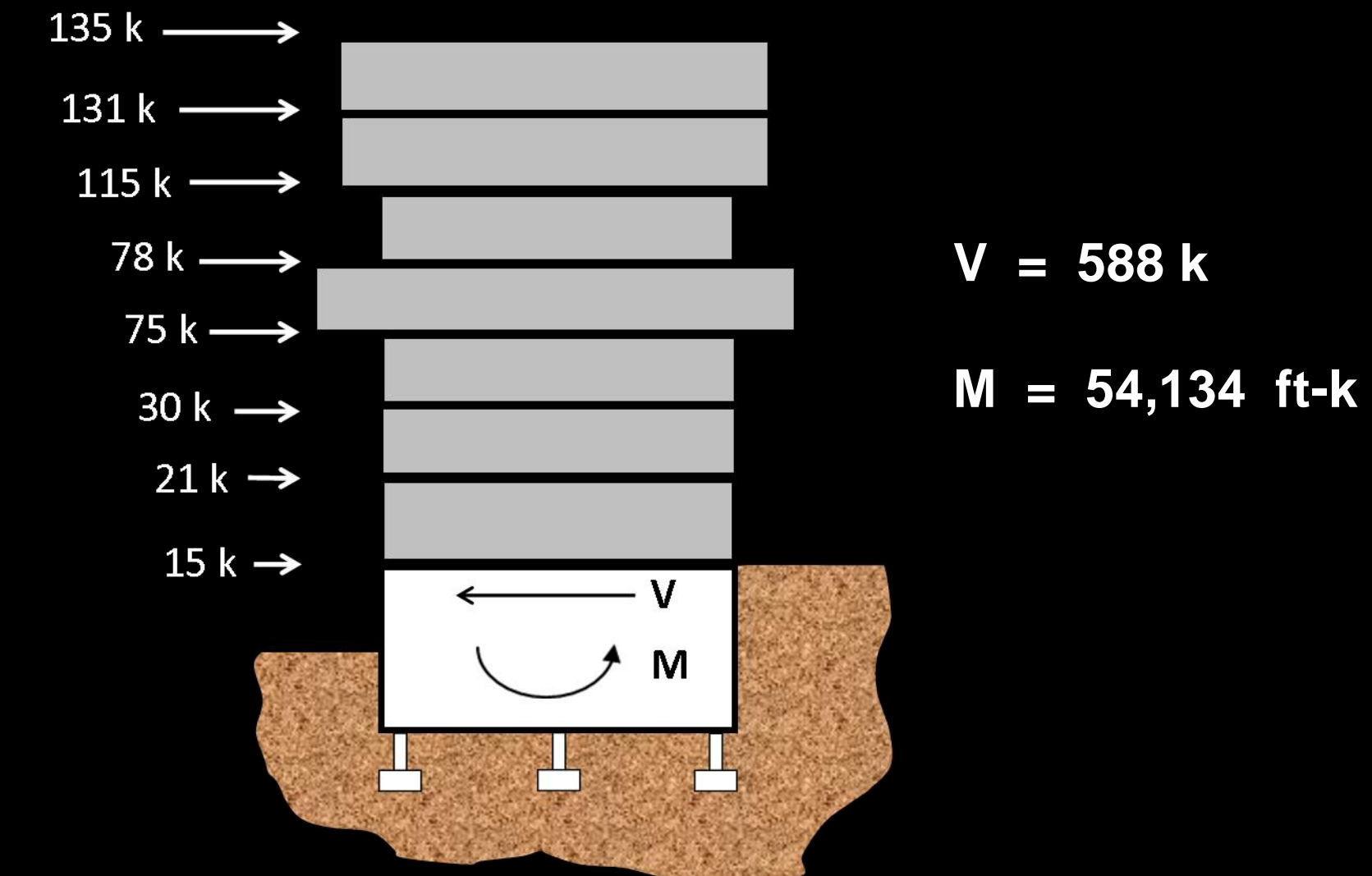
ETABS: 3D Truss





## Lateral Analysis

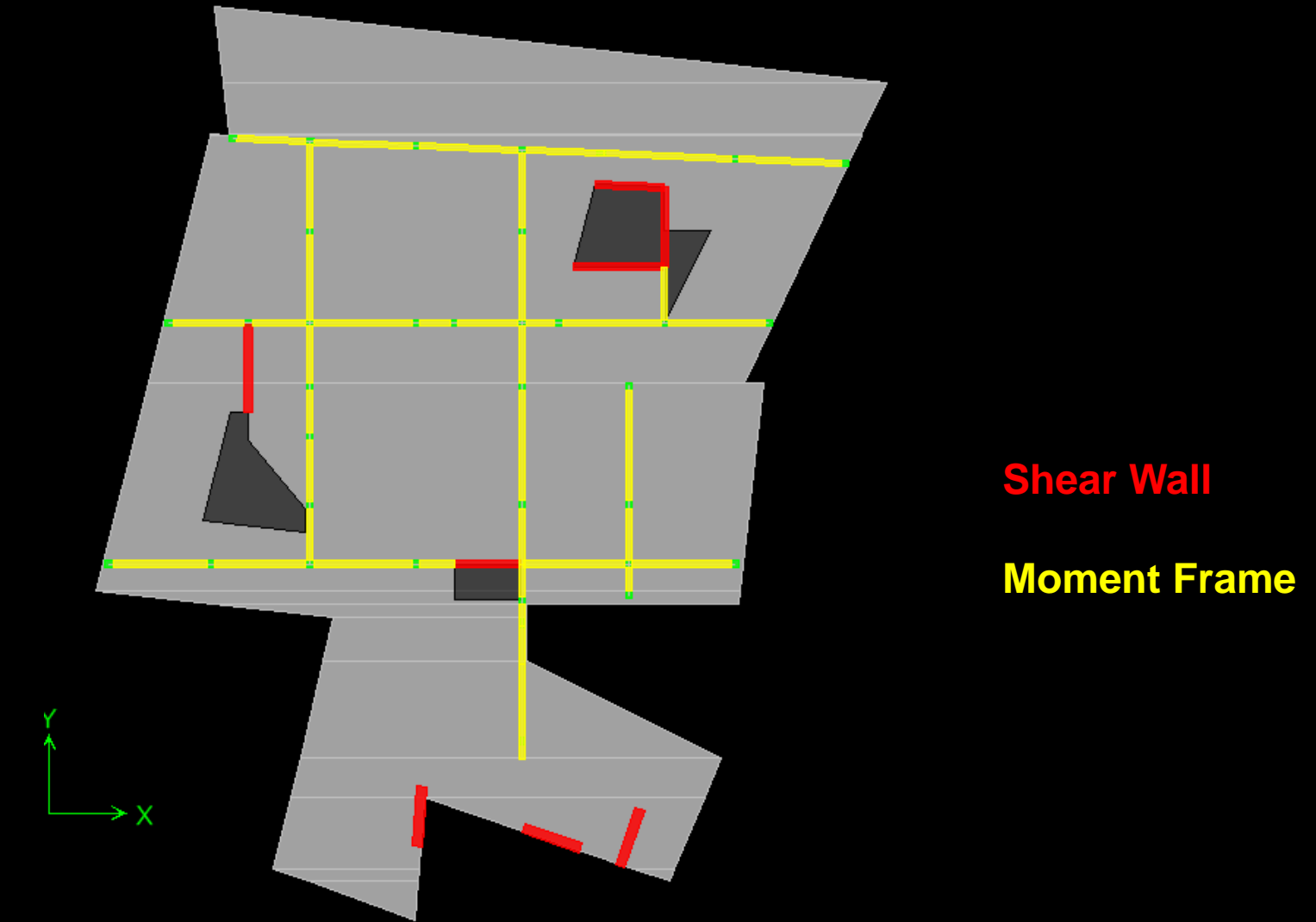
- **Seismic Design Category “B”**
- **ASCE7-05 §12.8** Equivalent Lateral Force Procedure
- **Shear Wall-Moment Frame Interactive System**
- **ASCE7-05 §12.3.2.1** Horizontal Irregularity
  - Type 1b: Extreme Torsional Irregularity
- **ASCE7-05 §12.5:** Directional Loading
  - Orthogonal Loading Only





## Lateral System

- **Shear Wall-Moment Frame Interactive System**
  - **Shear Walls**
    - 4 - North/South
    - 4 - East/West
  - **Moment Frames**
    - 4 - North/South
    - 3 - East/West



Shear Wall

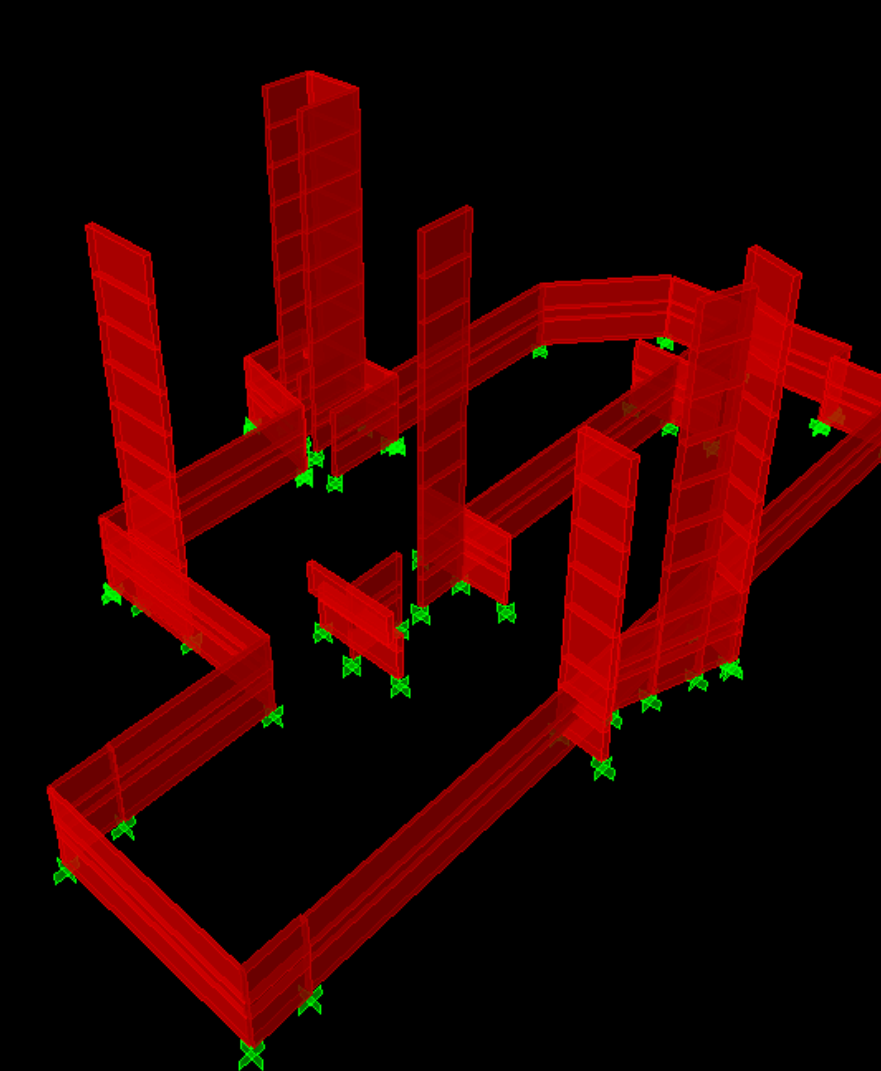
Moment Frame



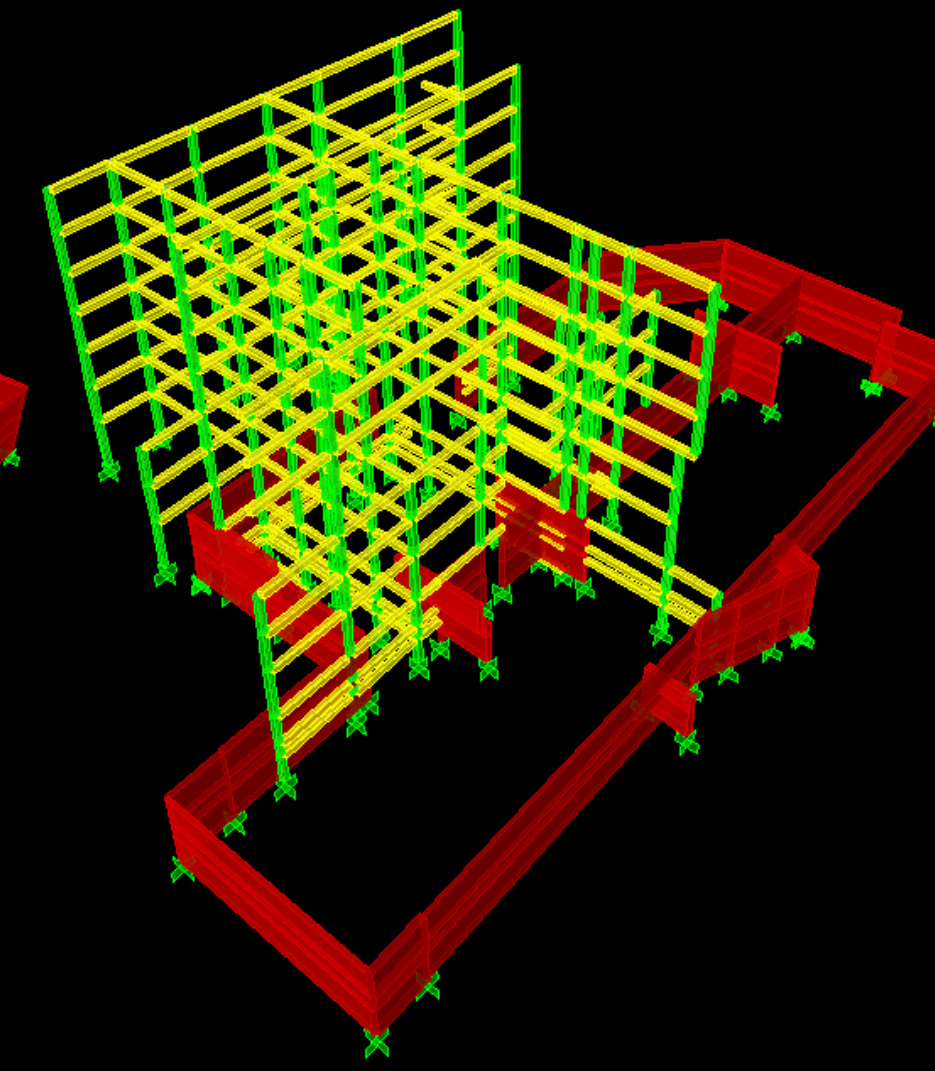


## ETABS Analysis

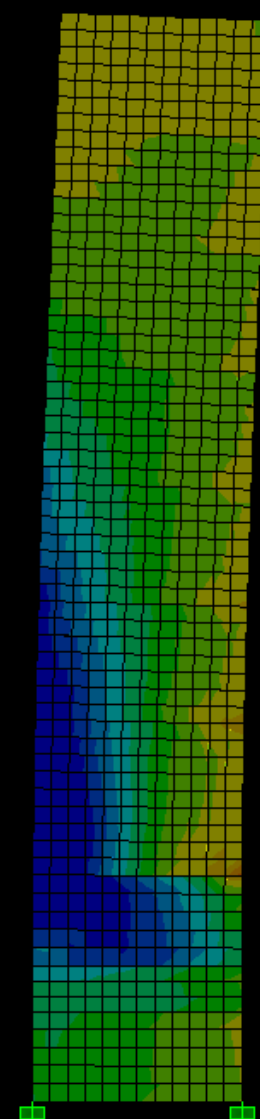
- **Shear Walls**
  - 8000 psi
  - Membrane
  - $I = 0.35I_g$
- **Moment Frames**
  - 6000 psi
  - $I_{col} = 0.7I_g$
  - $I_{bm} = 0.35I_g$
  - Rigid End Offset = 0.5
- **Rigid Diaphragm**



ETABS: 3D Shear Walls



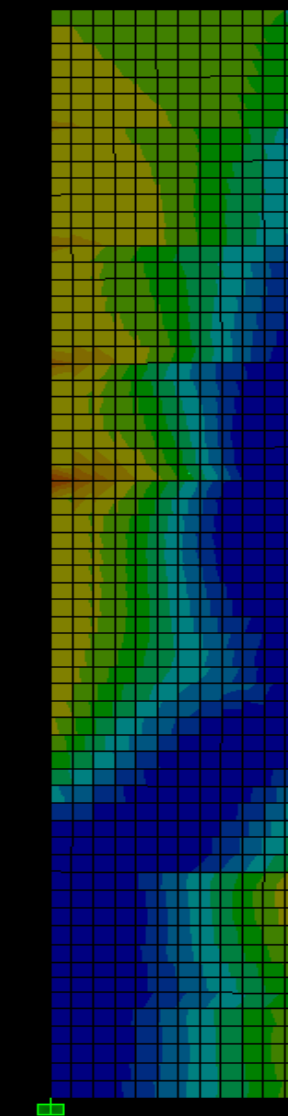
ETABS: 3D Moment Frames



### Shear Wall G7 (N/S)

- **Design Values**

- $V_{\max} = 157.5 \text{ k}$
- $M_{\max} = 9642 \text{ ft-k}$
- $P_{\max} = 1704 \text{ k}$

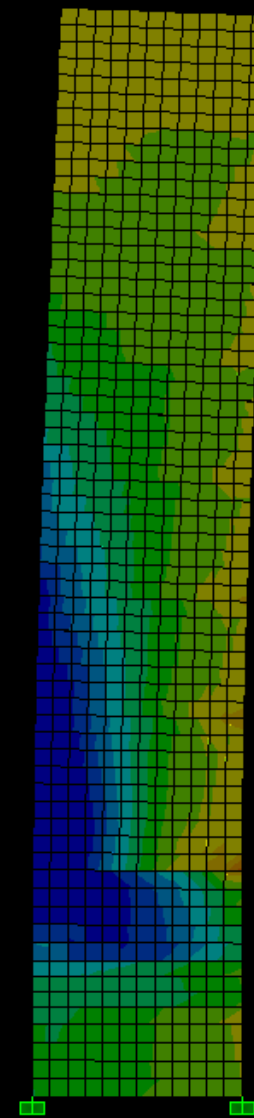


### Shear Wall GG (E/W)

- **Design Values**

- $V_{\max} = 196 \text{ k}$
- $M_{\max} = 15,000 \text{ ft-k}$
- $P_{\max} = 960 \text{ k}$

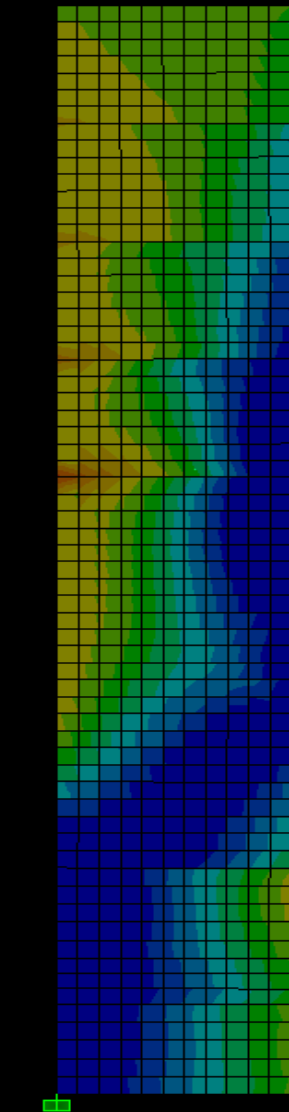




### Shear Wall G7 (N/S)

Flexural Reinforcement				
As,min	5.8	in2		
As	17.78	in2	Try (14) #10's	
As,max	54.6	in2		
$\epsilon_s =$	0.01848		>.00207	OK!
			>.005	$\phi = 0.9$
$\phi Mn$	10,997	ft-k	> 9641.7	OK!

**(14) #10s @ Ea. End**



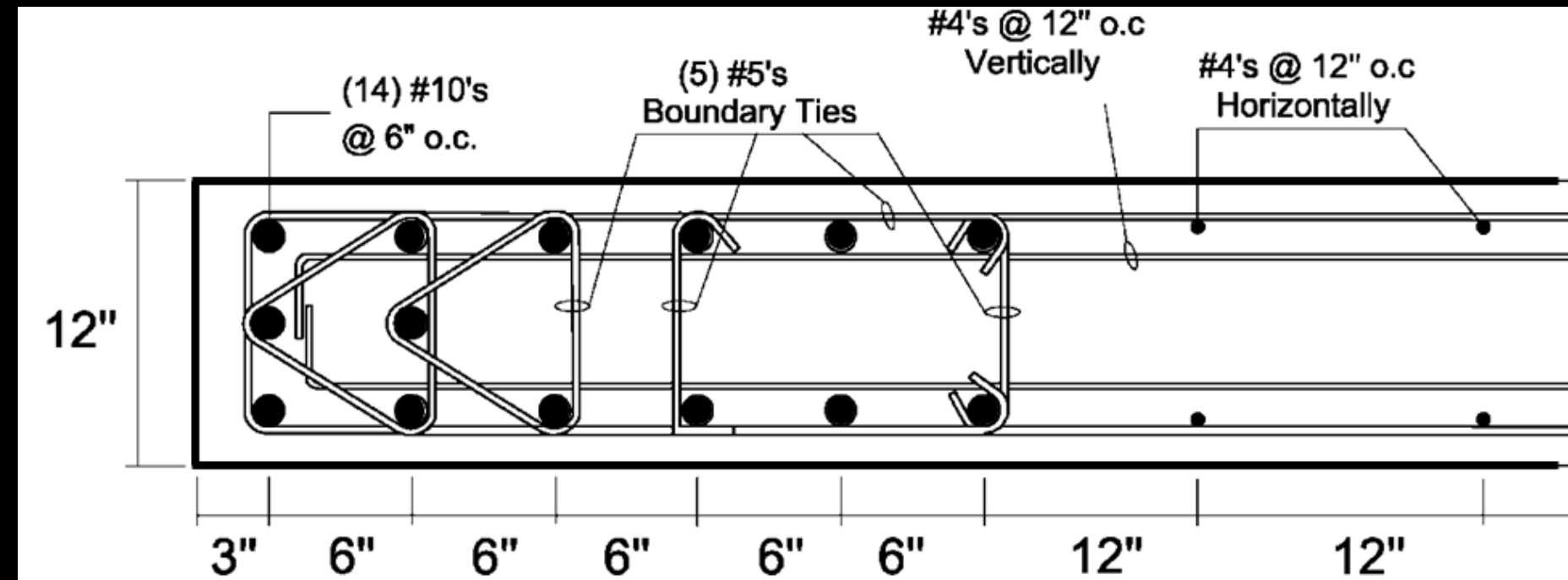
### Shear Wall GG (E/W)

Flexural Reinforcement				
As,min	10.3	in2		
As	17.78	in2	Try (14) #10's	
As,max	72.7	in2		
$\epsilon_s =$	0.0261		>.00207	OK!
			>.005	$\phi = 0.9$
$\phi Mn$	15,158	ft-k	> 15,000	OK!

**(14) #10s @ Ea. End**



### Shear Wall Design

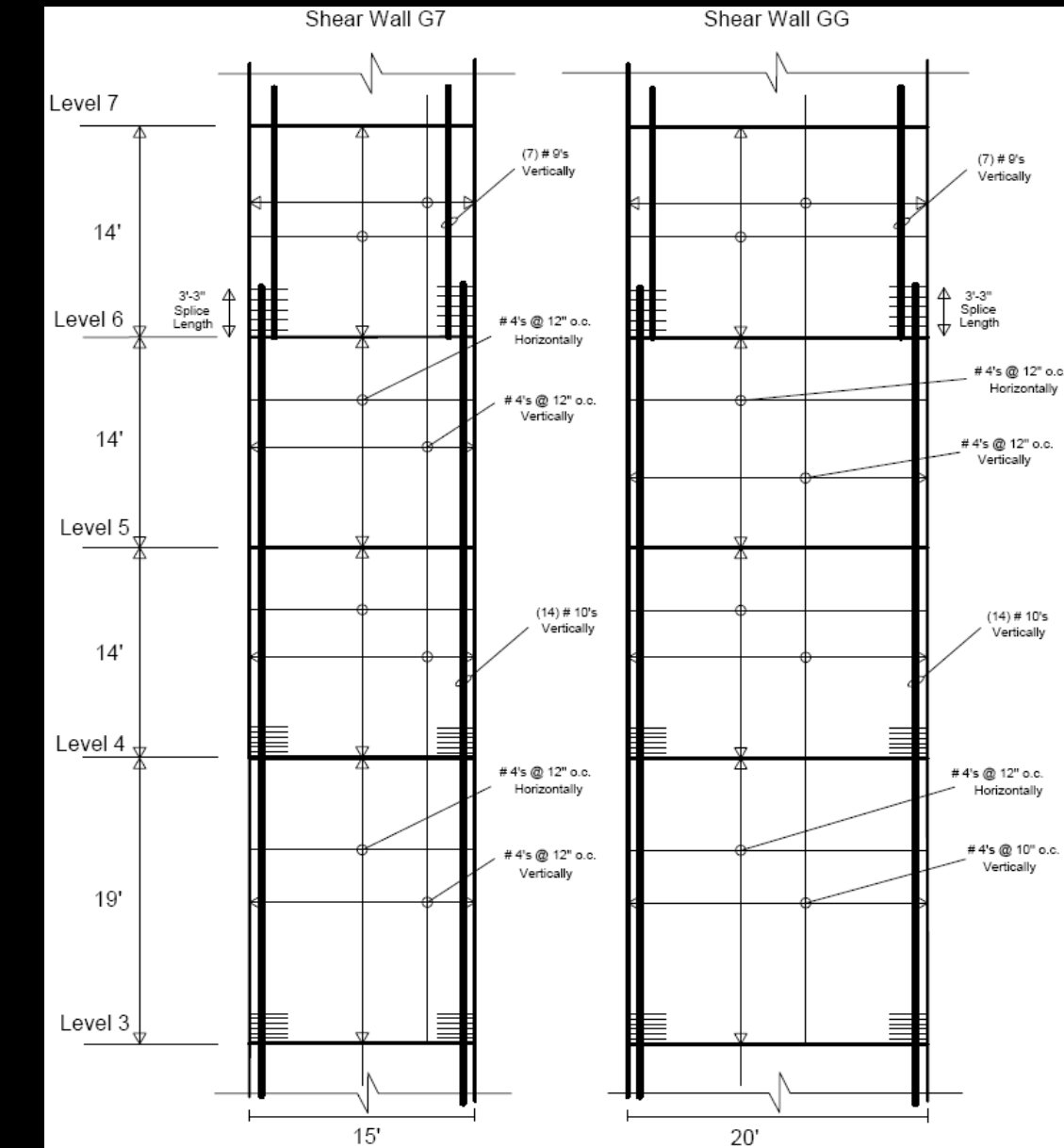
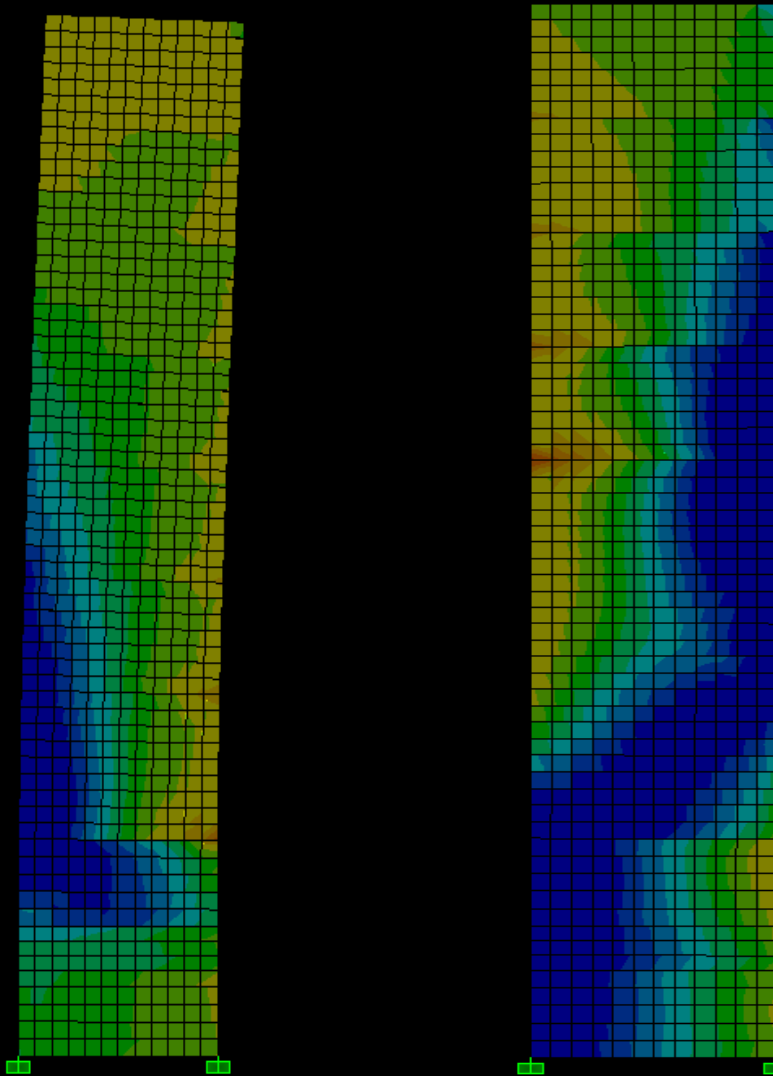


Shear Wall Design Section



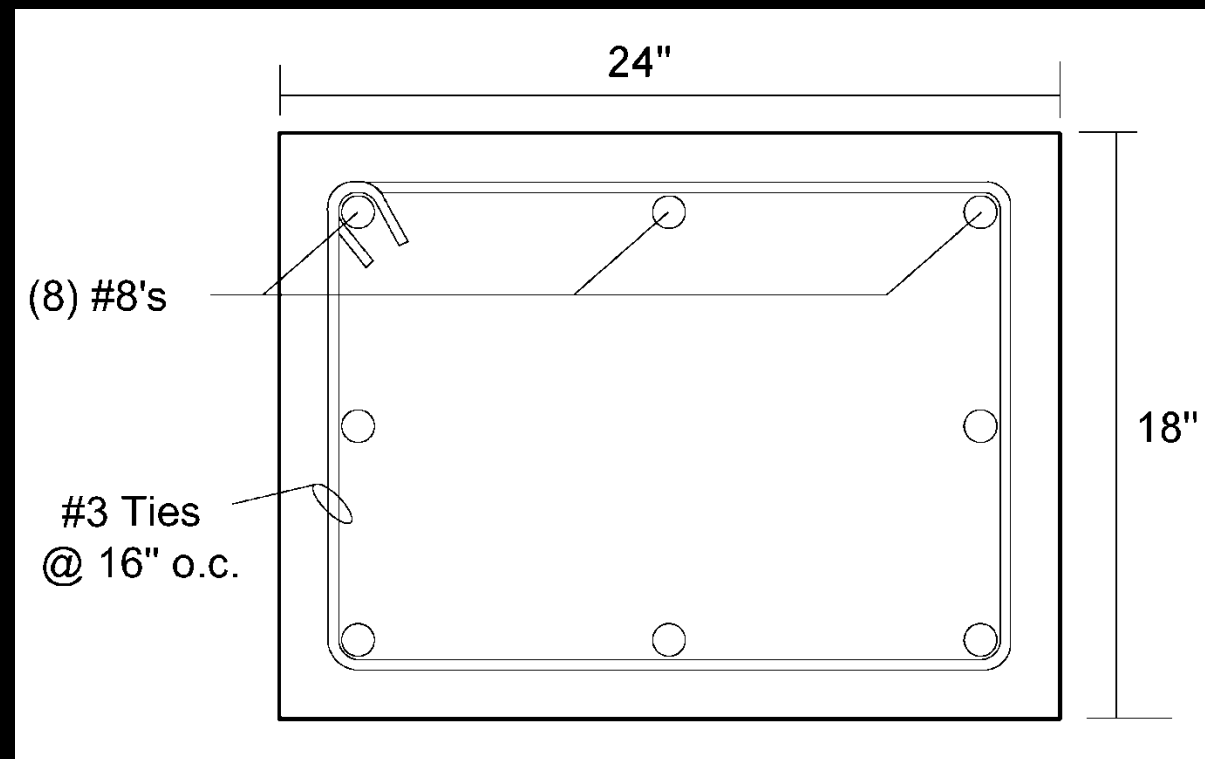


## Shear Wall Design

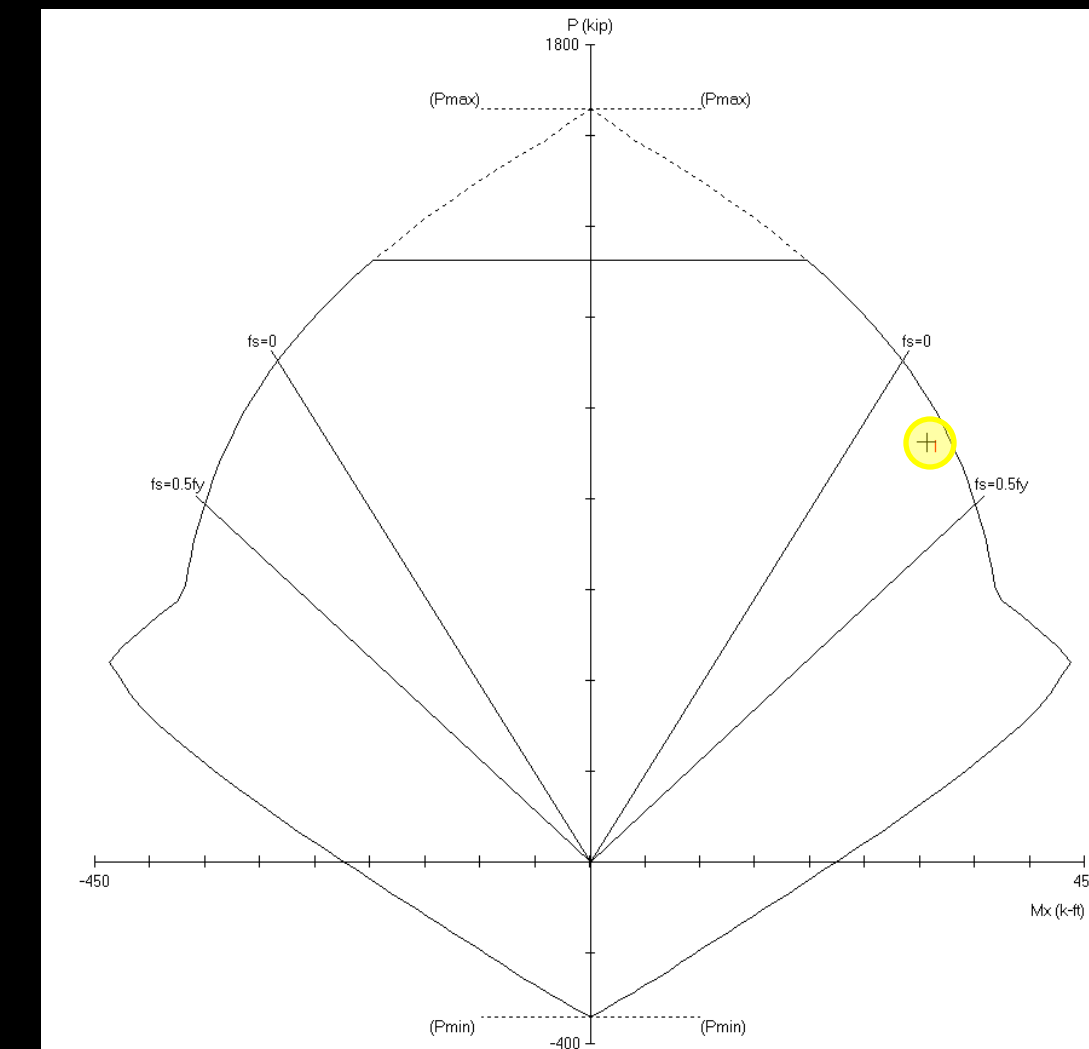




## Moment Frame Column Design



Moment Frame Column Design Section

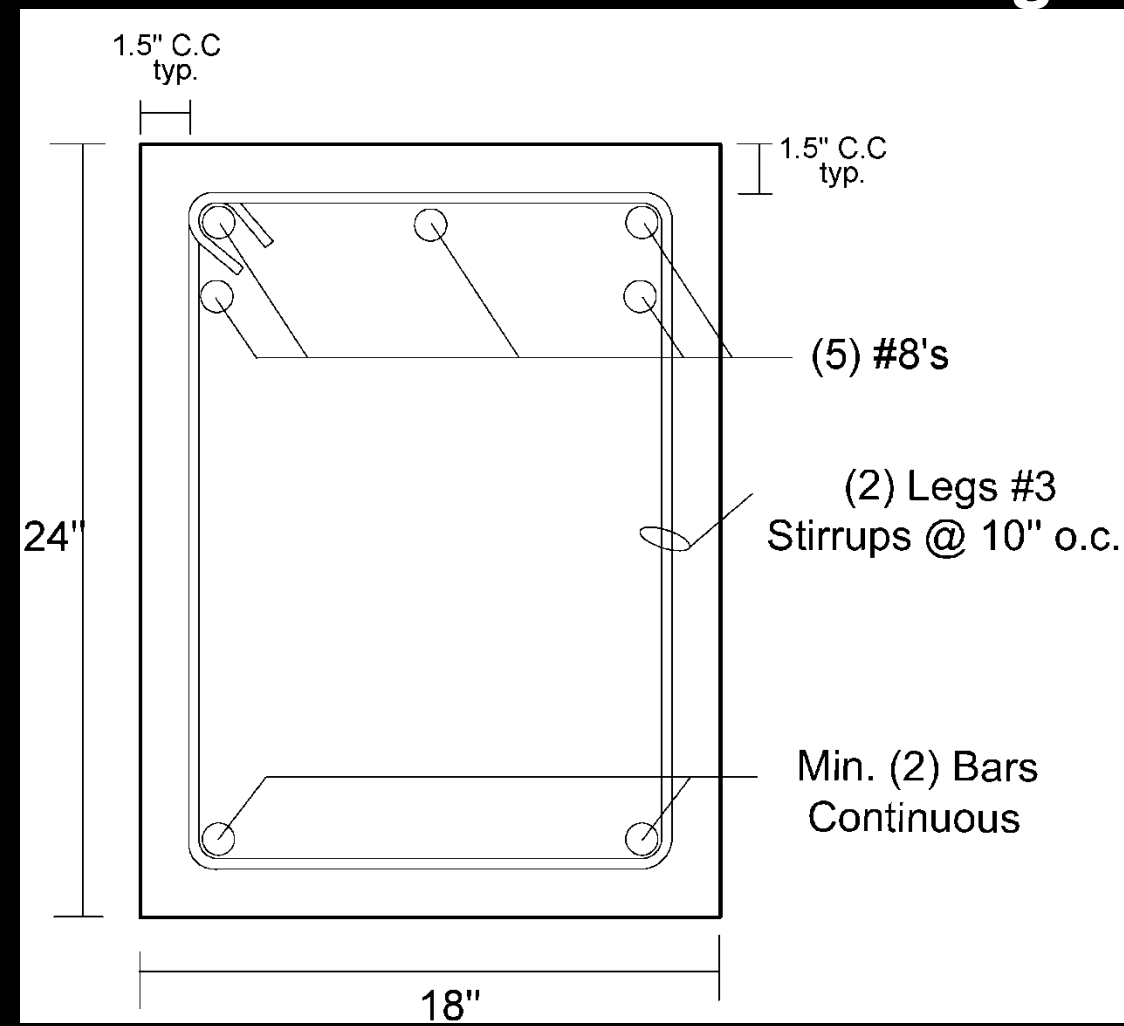


spColumn: Interaction Diagram

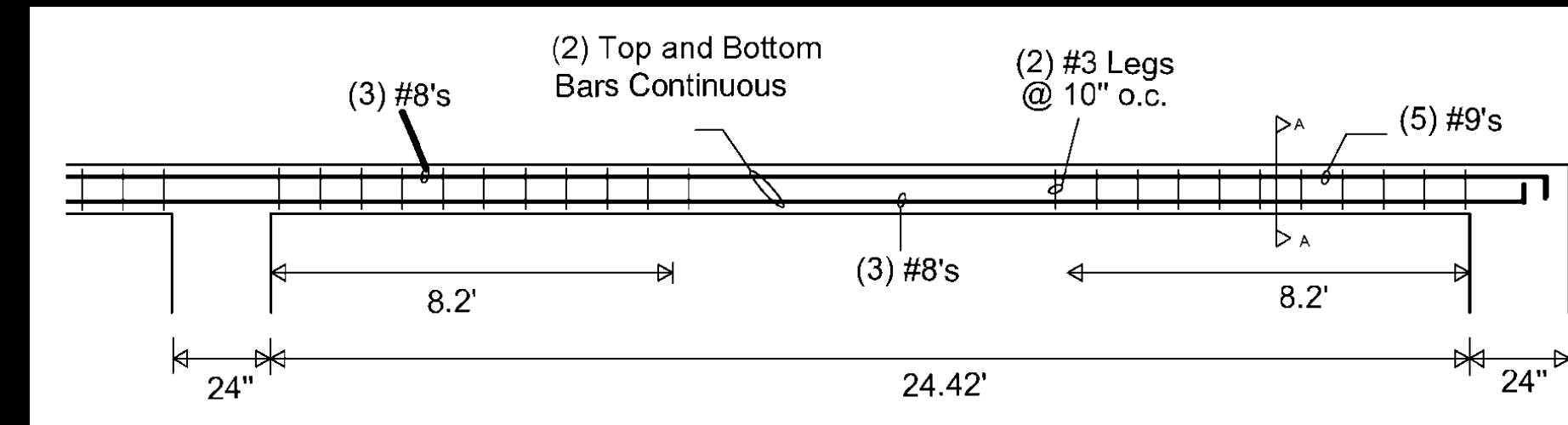




### Moment Frame Beam Design



Moment Frame Beam Design Section



Design Beam Section: Exterior Span



## Schedule Analysis

- **Existing**
  - **Start:** May 5<sup>th</sup>, 2008
  - **Finish:** November 11<sup>th</sup>, 2008
- **Proposed**
  - **Start:** May 5<sup>th</sup>, 2008
  - **Finish:** August 12<sup>th</sup>, 2008
- **Comparison**
  - 2 months more efficient





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  - **Finish:** August 12<sup>th</sup>, 2008
- **Comparison**
  - 2 months more efficient

## Cost Analysis

- **Existing**
  - \$ 4,486,006
- **Proposed**
  - \$ 5,281,312
- **Comparison**
  - ( \$ 795,306 )

Introduction

Existing

Problem

Solution

Gravity

Lateral

**Construction**

Conclusion

Questions?







## Conclusions

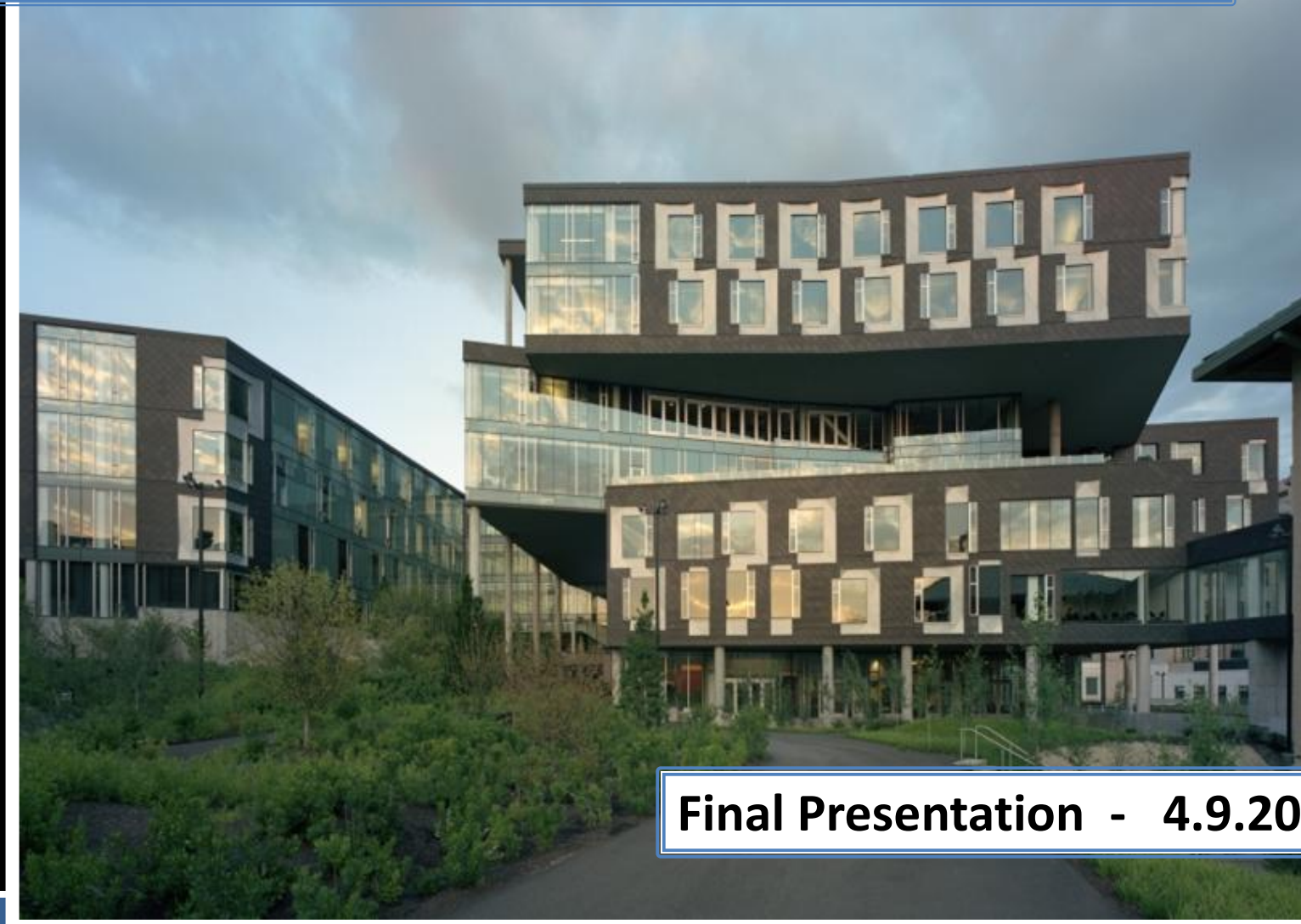
- **Design**
  - Strength and Serviceability Requirements
    - Gravity System **OK!**
    - Lateral System **OK!**
- **Construction**
  - Incurred cost **Plausible**
  - Shortened Schedule **OK!**

## Acknowledgements

- **Dr. Boothby**
- **Professor Parfitt**
- **AE Faculty**
- **Matt Wetzel of PJ Dick**
- **Bill Hawk of Graziano Construction**
- **Family**
- **Friends for reinforcing core structural concepts**



## Questions and Comments?



Final Presentation - 4.9.2012

Chris Dunlay

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