Christopher McCune Structural Option 2006 Senior Thesis Penn State Architectural Engineering

Eight Tower Bridge Conshohocken, Pennsylvania



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✓Introduction

Outline

Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Presentation Outline

Project Background
 Project History
 Architectural Description

•Original Structure •System Description •Problem Development

•Proposal •Design Objectives •Design Criteria •Structural Design •Gravity System •Lateral System

Construction Management
Cost Analysis
Schedule Comparison

•Final Conclusions

•Questions



✓Outline Project Background

96

✓Introduction

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Project Background





✓Introduction

√Outline

Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Project Background

Basic Information •Conshohocken, Pennsylvania •historic area •16 stories+ mechanical penthouse •345,000 square feet •Constructed from February 2001 to April 2002 •\$43 million total project cost

Key Players

•<u>Owners</u>: Oliver Tyrone Pulver Corporation & Brandywine Realty Trust •<u>Architect</u>: Skidmore, Owings & Merrill •<u>Structural</u>

<u>Engineers</u>: Skidmore, Owings & Merrill •<u>Mechanical Engineers</u>: Jaros, Baum & Bolles •<u>Steel Contractor</u>: Grossi & Sons Steel •<u>General Contractor</u>: R.M. Shoemaker







✓Introduction

√Outline

Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Project Background

Project History

•Building is part of a series of similar office projects •Oliver Tyrone Pulver has worked with both SOM and Jaros, Baum & Bolles before

Description

•Multi-tenant office tower
•Open office floor plan, central core construction
•21,500 square feet/floor
•Ground level doubles as entry and parking area

Architecture

Features two story entry lobby
Façade is "signature" precast concrete panel with stone trim and tinted green glass
Corner terraces located on 16th floor





✓Introduction

96

✓Project Background

✓Outline

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Original Structure

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓ ProjectBackground

Original Structure

Proposal

Structural Design

Construction

Management

Final Conclusions

Questions



Foundation

- 4000psi 16" diameter auger cast piles with caps
- 4'3" mat foundation at core
- •Connected by various size grade beams



Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background

Original Structure

Proposal

Structural Design

Construction

Management

Final Conclusions

Questions



Floor System

Typical Floor Layout





Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Typical Bay Plan •Bay Size: 28' x 44'4" •Beams: W18x40 spaced @ 9'4" •Interior Girders: W18x50/86 •Exterior Girders: W21x44



Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Floor System

Flooring System

- •Floor beams act in full composite with slab
- •ASTM 992, Grade 50 W18x40 beams, 1³/₄" upward camber
 •3¹/₄" lightweight concrete slab over 2" metal deck with ³/₄" shear studs

Total System depth: 23¹/₄"



Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Columns

Columns •W14 shapes ranging from 550lbs/ft to 90 lbs/ft •Designed for floor to floor height of 12'1", spanning two stories

Floor to ceiling height: 9'0"
•23¹/₄" floor system depth
•13³/₄" mechanical plenum space





✓Introduction

✓Outline

✓Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Lateral System

Combination of braced and moment frames
Braced frames found at core of building
Moment frames found at building perimeter







✓Introduction

- **√O**utline
- ✓Project Background

Original Structure

- Proposal
- Structural Design
- Construction Management
- **Final Conclusions**
- Questions



Original System

•3D rendering of steel frame

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

√Outline

✓Project Background

Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Problem Development

•Eight Tower Bridge has been designed and built as a steel structure

•Steel is a popular material for office buildings in the Philadelphia area

•Most engineers will consider multiple designs

COULD THIS BUILDING BE DESINGED AS A CONCRETE STRUCTURE?



✓Introduction

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Proposal

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Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background
- ✓Original Structure

Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Proposal

Design an alternate concrete structural system with the following objectives:

Maintain an open office layout, free of column obstruction
Maintain an overall floor system depth less than or equal to the existing 23¹/₄" depth
Limit floor deflection to 1/360

•Limit lateral building drift to 1/400

•Minimize cost and schedule

Design the system under the following criteria and codes •IBC 2000 •ASCE7-2002 •ACI 318-05





✓Introduction

- ✓Outline ✓Project
- Background ✔Original
- Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions





Structural Design

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√Outline**
- ✓ Project
 Background
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Where to begin?

Floor system controls design •Floor weight determines column size •Floor to floor height can affect lateral system

Concrete options?

•Two-way flat plate
•Two-way flat plate with drop panels
•Regular reinforced beams (T-beam)
•Post-tensioned system

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- ✓Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Floor System Design

Two alternate concrete floor systems were designed

System #1: One-way post-tensioned beam and slab

- •Post-tensioning in beams only
- •6" regular reinforced concrete slab

•20"x20" typical beams spaced 14' o.c. (22x20 long span girders)

•5000 psi concrete







✓Introduction

- **√O**utline
- ✓Project Background
- ✓Original Structure
- ✓Proposal
- Structural Design
- Construction Management
- **Final Conclusions**
- Questions



System #1- PT Beam and Slab

Longitude tendon plan

- •½"-7-wire tendons used
- •Tendons stressed to 26.6kips/tendon (includes losses)

	18S F=482	18S F=482	12S F=321	14S F=375	12S F=321	16S F=428	12S F=321	16S F=428	16S F=428	18S F=482	18S F=482	F	
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✓Introduction

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



System #1- PT Beam and Slab



Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



System #2- PT Beam and Slab

System #2: One-way post-tensioned beam and post-tensioned slab •6" post tensioned concrete slab with 4 tendons @ 6' o.c •18"x30" typical beams spaced 28' o.c. •5000 psi concrete

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Eight Tower Bridge Conshohocken, Pennsylvania

-0.07

0

0.07

0.14

0.21

0.28

0.35

0.42

0.49



✓Introduction

- ✓Outline
- ✓Project Background
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Deflection Calculations

RAM Concept sustained service load deflection plan for system #2







✓Introduction

- **√O**utline
- ✓Project Background
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Column Design

Columns were designed using two programs and hand calculations •Axial load development by hand •PCA COL with loads and estimated moments •Finalize design using ETABS and lateral load analysis









✓Introduction

- **√O**utline
- ✓Project Background
- ✓Original Structure
- ✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Shear Wall Design

A series of eight, 12" thick shear walls were designed to resist lateral loads using ETABS

С

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✓Introduction

- **√O**utline
- ✓Project Background

✓Original Structure

✓Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Shear Wall Design

Deflections were calculated under each of the four wind load cases in ASCEE7-02, as well as both seismic directions

Lateral Load Deflection Summary								
	$\Delta \mathbf{x}$	$\triangle \mathbf{Y}$						
Wind Case 1X	1.76"	-						
Wind Case 1Y	-	1.65"						
Seismic X	4.66"	-						
Seismic Y	-	4.55"						
Wind Case 2X	1.32"	-						
Wind Case 2Y	-	1.23"						
Wind Case 3	1.29"	1.23"						
Wind Case 4	0.98"	0.93"						
Wind Case 3 Wind Case 4	1.29" 0.98"	1.23" 0.93"						

L/400 = (207^{*}12["])/400=6.21["]

Maximum is L/533







✓Introduction

- **√O**utline
- ✓Project Background
- ✓Original Structure
- ✓ Proposal

Structural Design

Construction Management

Final Conclusions

Questions



Shear Wall Design

Summary of forces in each wall

Shear Wall Forces under Seismic X Loading (kips)												
	Wall											
Level	Α	В	С	D	E	F	G	Н				
Level 2	99.95	N/A	99.36	97.15	N/A	97.77	266	264.8				

H

G

C

A

D

F

Total Base shear under seismic-x loading= 925kips

Shear wall reinforcement was calculated through ETABS •At base, ρ =.0234 •At top, ρ =.0025



✓Introduction

96

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal
- ✓Structural Design

Construction Management

Final Conclusions

Questions



Construction Management

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓Project Background
- ✓Original Structure
- ✓Proposal
- ✓Structural Design

Construction Management

Final Conclusions

Questions



Cost Comparison

A cost estimate was obtained for the original steel system, and the alternate concrete system using both floor systems using RS Means 2005.

The estimate received from Grossi & Sons was in "2001 dollars". Therefore, all estimates using RS Means 2005 were converted into "2001 dollars" using the equation below:

 $(F/P) = 1/(1+i)^n$

F= future value P= present value i= interest rate n= time period

	Steel Estimate For Eight Tower Bridge										
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	Steel	QIY									
S	Beams	3200									
	Columns	350									
	Angle Braces	280		\$2,300,000							
	Moment Conncetions	834									
	Instalation			\$1,125,000							
			TOTAL	\$3,425,000							
	Decking	QTY									
	Metal Deck										
	Shear Studs	48000		\$450,000							
	Instalation			\$525,000							
			TOTAL	\$975,000							
	Concrete	QTY									
	4000 p si	3451 C.Y.		\$310,590							
	Placing			\$103,865							
			TOTAL	\$414,455							
			TOTAL	\$4,814,455							





✓Introduction

96

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal
- ✓Structural Design

Construction Management

Final Conclusions

Questions



Comparison Summary

Superstructre System Summary											
	Cost	persquare	e foot	Duration (weeks)							
System		Crane	Pump		Crane	Pump					
Steel	\$13.94	-	-	28	-	-					
Concrete System #1		\$14.51	\$13.75	-	28.14	23.7					
Concrete System #2		\$14.21	\$13.51		28.14	23.7					

•All costs are in "2001 dollars"

•The total cost per square foot is based on a 345,000 sq. ft. building

•The estimate does not include foundations

•Schedule and estimate includes shoring costs for concrete



✓Introduction

96

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal
- ✓Structural Design
- ✓ Construction Management

Final Conclusions

Questions



Final Conclusions

Eight Tower Bridge Conshohocken, Pennsylvania



✓Introduction

- **√**Outline
- ✓ Project
 Background
- ✓Original Structure
- ✓Proposal
- ✓Structural Design
- ✓ Construction Management

Final Conclusions

Questions



Final Conclusions

Advantages

Both alternate concrete floor system perform better in regards to deflection
Both floor systems either reduce the overall building height, increase mechanical plenum space, increase floor to ceiling height, or a mix of all 3
The concrete lateral system will deflect more under seismic loads, but less under wind loads, which it will see more often

However...

Steel building is cheaper per square foot
Philadelphia is not extremely booming concrete market
Special selection of PT contractor required
All parties involved are experienced with steel structural system
Owner has built similar buildings

It is suggested to use the original steel design



✓Introduction

96

- **√**Outline
- ✓ ProjectBackground
- ✓Original Structure
- ✓Proposal
- ✓Structural Design
- **✓**Construction
- Management
- ✓ Final Conclusions

Questions



QUESTIONS?