

The Regent 950 N. Glebe Road

Arlington, VA



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Presentation Outline

- Introduction to The Regent
- Overview of the Existing Structural System Design
- Proposal Summary and Design Goals
- Structural Depth Study: CIP Concrete Design
- Construction Management Breadth Study: Cost and Schedule Analysis
- Mechanical Breadth Study: Mechanical Layout Impact Analysis
- Structural System Design Comparisons (Steel vs. Concrete)
- Conclusions
- Acknowledgements and Credits

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Introduction to The Regent

- Location: 950 N. Glebe Road, Arlington, VA
- Architecture
 - 12 stories above grade 265,243 SF
 - Office (Levels 2-12)
 - Retail (Level 1)
 - Floor to Floor Height = 13'
 - Floor to Ceiling Height = 9'
 - 3 levels of parking below grade -158,889 SF
 - Office levels are open floor plans with a typical central core
 - Height ≈ 180 FT
- Construction Management
 - \$32,000,000
 - Final completion 9-5-06



Lobby

Cooper Carry Architects



Eastern Elevation

Cooper Carry Architects

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Existing Structural System

- Parking Garage Structure (below grade)
 - Cast-in-place concrete columns
 - Columns on a 30' x 30' grid
 - Flat slabs with drop panels

Superstructure

- Long-span (46'/30') composite steel beams @ 10' o.c. with 3.25" lightweight slab on 3" composite metal deck
- Bay sizes: 30' x 30' and

46' x 30'

- Typical beam and girder sizes: W18's, W16's, and W21's
- Typical column sizes: W14's



Composite Beam Floor System

Existing Structural System

- Lateral Force Resisting System
 - N/S (2) Braced frames
 - 30' long
 - Run the entire height of the building
 - E/W (3) Braced frames
 - 30' long
 - Run the entire height of the building
- Foundations
 - Square spread footings
 - Sizes ranging from 4' x 4' to 9' x 9'
- Building Code IBC 2000



Braced Frame



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Proposal Summary and Design Goals

- Maintain design team's goals
 - Minimize floor system depth
 - Maximize floor to ceiling height
 - Meet maximum height restriction \approx 180'
 - Open floor plan with minimal column interruptions (spec office building)
 - Minimal building costs
 - Quick construction schedule
- Goal: Design an alternative system to meet or exceed most of these goals

Proposal Summary and Design Goals

- Proposed Structural System Design:
 - Gravity: CIP concrete system using wide-module joists
 - Lateral: Shearwalls
 - Can accommodate larger spans 46'/30'
 - Concrete system depth potentially less than or similar to steel system
 - Concrete system costs may be less than today's higher steel material costs
 - Concrete systems are common in Washington D.C. area - labor, materials, equipment more readily available

Proposal Summary and Design Goals

- System Comparisons
 - CIP concrete system more efficient for The Regent?
 - Any advantages gained by using a CIP concrete system?

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Structural Depth Study Overview

- Scope (Superstructure and Foundations)
 - CIP Joist Designs
 - CIP Girder Designs
 - CIP Column Designs
 - CIP Shearwall Designs
 - Representative Spread Footing Designs
 - Roof Design
- Codes and Code Load Requirements
 - IBC 2000
 - ASCE 7-02
 - Live Loads reduced where applicable

Structural Depth Study Overview

- Design Goals and Assumptions
 - Joists, girders, and columns cast monolithically
 - Slab thickness = 4.5''
 - Joist and Girder Deflection Limits
 - Total Load L/360
 - Live Load L/480
 - fy = 60 ksi
 - **–** f'C
 - Joists, Girders, Shearwalls = 4,000 psi
 - Columns = 5,000 psi (minimize column sizes)
 - Foundations = 3,000 psi
 - ACI 318-02 used for cast-in-place member designs
 - Keep existing column layout open floor plan



CIP Joist Designs

 CIP wide module joists span in the East/West direction

- Analysis
 - Design Moments and Shears
 - Moment distribution with live load pattern loading

CIP Joist Layout Plan







40" Forms	40" Forms
8" ribs	8" ribs
4.5" Slab	4.5" Slab
24" Joist Depth	16" Joist Depth
Concrete System Floor Depth = 28.5"	Concrete System Floor Depth = 20.5"
Steel System Floor Depth = 24.25"	Steel System Floor Depth = 22.25"



CIP Girder Designs

CIP Girder Analysis

- Analysis
 - Moments
 - ACI 318-02, Section 8.3.3 moment equations
 - Portal Analysis 25% Earthquake moments
 - Shear
 - ACI 318-02, Section 8.3.3 shear equations
 - Torsion
 - Joist FEMs







Typical Floor Bay Plan and Section Plan



Section





Typical Exterior Joist Detail



Typical Interior Joist Detail





CIP Column Designs

CIP Column Design Goals and Assumptions

- Keep same column layout
- Columns designed to take 25% of the seismic load -ASCE 7-02, Chapter 9, Section 9.5.2.2.1
- Some interior columns are boundary elements for shearwalls
- Reinforcement placed at equal spacings
- Columns designed in 3 sections at the changes in floor plan
 - Levels 1-5
 - Levels 6-9
 - Levels 10-12



CIP Column Analysis Methods

- Design Moments
 - Live and Dead Moments
 - North/South Moments: Girder moments
 - East/West Moments: Joist FEMs
 - 25% Earthquake Moments
 - Portal Analysis
- Axial Loads
 - Live and Dead Loads
 - Tributary area

CIP Column Design Procedures

 PCACOL was used for the design for each of the three column sections for each column



Column Designs for Levels 1-5



CIP Column Designs

Column Designs for Levels 6-9



CIP Column Designs

Column Designs for Levels 10-12





CIP Shearwall Designs

CIP Shearwall Location Plan



CIP Shearwall Analysis Methods

- Trial shearwall size – 8" (typical)
- An ETABS model used for analysis of 8" shearwall designs
- Allowable total building deflection = H/400 or 5.40"



CIP Shearwall Deflections

8" Shearwalls

Wall	Max	xΔx	Max ∆y		Max Δz	
	E/S	W/N	E/S	W/N	E/S	W/N
1	2.053982"	2.032261"	1.503888"	1.503888"	0.164363"	-0.161891"
2	2.053982"	2.032261"	1.547330"	1.547330"	0.012486"	-0.289300"
3	2.053982"	2.032261"	1.570137"	1.570137"	0.148668"	-0.150556"
4	2.053982"	2.053982"	1.526333"	1.547330"	0.424625"	0.012286"
5	2.032261"	2.032261"	1.526333"	1.547330"	0.137942"	-0.289300"

Total Building North/South Deflection $\approx 2" < 5.40"$ OK Total Building East/West Deflection $\approx 1.5" < 5.40"$ OK

CIP Typical Shearwall Design





Representative Spread Footing Designs

Representative Spread Footing Location Plan



Representative Spread Footing Designs

- Footing E-7 lateral force resisting and gravity column – both systems
- Footing E-9 interior gravity column both systems
- Allowable soil bearing pressure is 40 KSF



Representative Spread Footing Comparison: Steel System vs. Concrete System

E-7	Concrete System	10.5' x 10.5' x 51"
	Steel System	9′ x 9′ x 50″
E-9	Concrete System	9.5′ x 9.5′ x 45″
	Steel System	8′ x 8′ x 38″

NOTE: Concrete system footings are larger by 1.5' in each plan dimension.

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Construction Management Breadth Study Overview

- Scope
 - Cost and schedule analysis for each system
 - Typical lower level floor
 - Representative spread footings
- Reference
 - RS Means Building Construction Cost Data for 2006 was used for the cost and schedule analyses



Cost Analysis

Typical Floor Cost Summary Concrete System

Total Cost	Concrete		
	Material	Labor	Equipment
Joists/Slab	\$225,435	\$130,625	\$4,543
Girders	\$48,707	\$58,946	\$965
Columns	\$24,756	\$27,330	\$625
Shearwalls	\$8,534	\$11,484	\$365
Shoring/Reshoring	\$149,865	\$9,943	\$0
	\$457,297	\$238,328	\$6,498



Typical Floor Cost Summary Steel System

Total Cost	Steel		
	Material	Labor	Equipment
Slab on Deck	\$41,814	\$10,153	\$1,881
Metal Deck	\$41,468	\$10,428	\$728
Beams	\$160,851	\$9,998	\$4,937
Columns	\$74,396	\$964	\$631
Braced Members	\$22,447	\$1,149	\$659
	\$340,976	\$32,692	\$8,836

\$382,504

Spread Footing Cost Summaries

	Cost				
Footing	Material	Labor	Equip.	Total Cost	
E-7 (Concrete)	\$2,052	\$863	\$6	\$2,921	≈ \$6 00
E-7 (Steel)	\$1,592	\$722	\$5	\$2,319	difference
E-9 (Concrete)	\$1,583	\$701	\$5	\$2,289	≈\$850
E-9 (Steel)	\$966	\$464	\$3	\$1,433	difference



Schedule Analysis



Typical Floor Schedule Analysis Concrete System

Final Schedule	Concrete	
	# of Days	
Joists/Slab	30.31	
Girders	11.03	
Columns	6.36	
Shearwalls	3.38	
Shoring/Reshoring	6.93	
	58.01	
		-
	58	days



Typical Floor Schedule Steel System

Final Schedule	Steel	
		_
	# of]
	Days	
Slab on Deck	10.36	
Metal Deck	8.08	
Beams	3.30	
Columns	1.33]
Braced]
Members	0.45	
	23.52	
	24	days

Spread Footing Schedule			
	Footing	Schedule	
	E-7 (Concrete) E-7 (Steel)	0.74 days 0.62 days	
	E-9 (Concrete) E-9 (Steel)	0.66 days 0.40 days	

NOTE: Concrete system footings take longer to construct.

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Structural System Comparison Chart

	Steel System	CIP Concrete System
Floor System Depth	24.25" (46' span) 22.25" (30' span)	28.5" (46' span) 20.5" (30' span)
Floor to Floor Height	13′	13′
Floor to Ceiling Height	Interior Bay – 9' Exterior Bay – 9'	Interior Bay – 9' Exterior Bay – 8'-8"
Cost of Typical Floor	\$382,504	\$702,123
Material	\$340,976	\$457,297
Labor	\$32,692	\$238,328
Equipment	\$8,836	\$6,498
Typical Floor Schedule	24 days	58 days
Cost of Foundation for Lateral Resisting and Gravity Column	\$2,319	\$2,921
Cost of Foundation for Gravity Only Column	\$1,433	\$2,289
Foundation Size for Lateral Resisting and Gravity Column	9' x 9' x 50"	10.5' x 10.5' x 50"
Foundation Size for Gravity Only Column	8' x 8' x 38"	9.5′ x 9.5′ x 45″
Allowable Depth for Mechanical System	Interior Bay – 14.25" Exterior Bay – 12.25"	Interior Bay – 16" Exterior Bay – 8"
Typical Floor System Weight	46 PSF	119/95 PSF
Column Sizes	W14′s	18-36″ SQ

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Conclusions

- Steel System is determined to be the most economical and efficient system for The Regent in comparison to the CIP system
 - Cheaper material and labor costs
 - Significantly shorter schedule (24 days/floor vs. 58 days/floor)
 - Thinner floor depth to accommodate the mechanical system layout and floor to ceiling height goals
 - Lighter system (46 PSF vs. ≈100 PSF)
 - Smaller foundations (by 1.5' in each square dimension)

Concrete System Advantages

- Cheaper construction equipment costs
- Interior bay has thinner floor system depth and greater allowable depth for the mechanical system

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Acknowledgements and Credits

- Project Team
 - Steve Sanko Structural Design Group, Ltd.
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 - Katie Peterschmidt Cooper Carry Architects
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 - CAD files, specifications, renderings
 - Kevin Gunthert JBG Owner Representative
 - Permission to study The Regent

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Questions?