

APPENDIX G PILASTER CALCULATION

Masonry Pilaster

Worst Case Loading

$$P = 177 \text{ K}$$

$$m = 35.4 \text{ 'k}$$

$$\text{Total Size} = \frac{177000 \text{ lb}}{(35.4 \text{ psf})} = (472 \text{ in}^2) \approx 24'' \rightarrow \text{try } 24'' \times 24''$$

Use 4 #4's

$$A_s = .60 \times 4 = 2.40 \text{ in}^2$$

$$A_g = 24 \times 24 = 576 \text{ in}^2$$

$$F_s = 24000$$

$$h = 384''$$

$$r = .3D = 9.2$$

Axial Force Capacity

$$\begin{aligned} P &= (.25 f_m A_g + .65 A_s F_s) \left(1 - \left(\frac{h}{140r}\right)^2\right) \\ &= (.25 (1500)(576) + .65 (2.40)(24000)) \left(1 - \left(\frac{384}{140(9.2)}\right)^2\right) \\ &= 215.4 \text{ Kips} > 177 \text{ Kips} \quad \therefore \text{OK} \end{aligned}$$

Check Moment Capacity

$$A_s = 2.40 \text{ in}^2 \quad n = 21.5$$

$$d = 23.625 - 1.25 - 1.5'' = 21.87$$

$$\rho = \frac{A_s}{BD} = \frac{2.40}{(24)(21.87)} = .0046$$

$$P_n = .0983$$

$$K_c = \frac{P_n + \frac{1}{2} \left(\frac{1.25}{21.87}\right)^2}{P_n + \left(\frac{1.25}{21.87}\right)} = \frac{.0983 + \frac{1}{2} \left(\frac{1.25}{21.87}\right)^2}{.0983 + \left(\frac{1.25}{21.87}\right)} = .6399$$

PILASTER CALCULATION

Masonry Pilaster

Let $13.99'$ out of shell

$$t_e/D = 1.25/21.87 = .0571$$

$$j = 1 - \frac{t_e}{3} = .7867$$

$$jd = 19.2'$$

$$f_b = 500 \text{ psi}$$

$$f_{b2} = 500 \left(1 - \frac{1.25}{13.99}\right) = 455.3$$

Distance to resultant

$$1.25 \left(\frac{2(455.3) + 500}{3(955.3)} \right) = .615''$$

$$M_m = \frac{1}{2} (500 \text{ psi} + 455.3) (1.25 \times 112) (1.255) = 95''\text{k}$$

$$M_s = 2.40 \text{ in}^4 (24000 \text{ psi}) (21.87 - .615) = 102''\text{k}$$

$$95''\text{k} > 35.4''\text{k} \quad \text{OK}$$

use $24'' \times 24''$ pilaster

with 4 # 7's