



SOHO

HIGH RISE

CONDOMINIUMS

A STUDY OF LIGHTWEIGHT  
CONCRETE CONSTRUCTION

# Location and Architecture

JOSEPH MUGFORD

THESIS PRESENTATION

SPRING 2007



# LOCATION AND ARCHITECTURE

"This is a way to live at this time, in the poetry of the neighborhood. The emotion of architecture is linked to this harmony, to this relationship."

*Jean Nouvel, Architect*

SOHO, NY, NY

- Trendy urban Neighborhood of Downtown Manhattan
- Simple geometric shapes create areas of transparency, light and shadow
- Sleek horizontal and vertical lines provide a rigid architectural grid for the building façade, a combination of colored and transparent glass



# LOCATION AND ARCHITECTURE

## 15 Story Condominium Complex

Height: 175'

Base: 70' X 200'

Tower: 33' X 170'

Sub-Cellar Parking

Cellar Fitness and Spa

1<sup>st</sup> Floor Retail

10,000 SF

2-5: 27 Residential Units

11,800 SF/Floor

6-12: 13 Residential Units

4,650 SF/Floor

12-13: 2 Penthouse Units

6<sup>th</sup> Floor Roof Terrace

## Project Team

Owner: WXIV/Broadway Grand

Architect of Record: SLCE Architects

Design Architect: AJN Ateliers Jean Nouvel

Structural Engineers: Gilsanz.Murray.Steficek.LLP

MEP Engineers: Cosentini Associates

Curtain Wall Consultants: Gilsanz.Murray.Steficek.LLP

Landscape Architect: Thomas Balsley Associates

Lighting Consultant: Johnson Schwinghammer Inc.

General Contractor: Pavarini Construction Co. Inc.



# Existing Structure

JOSEPH MUGFORD

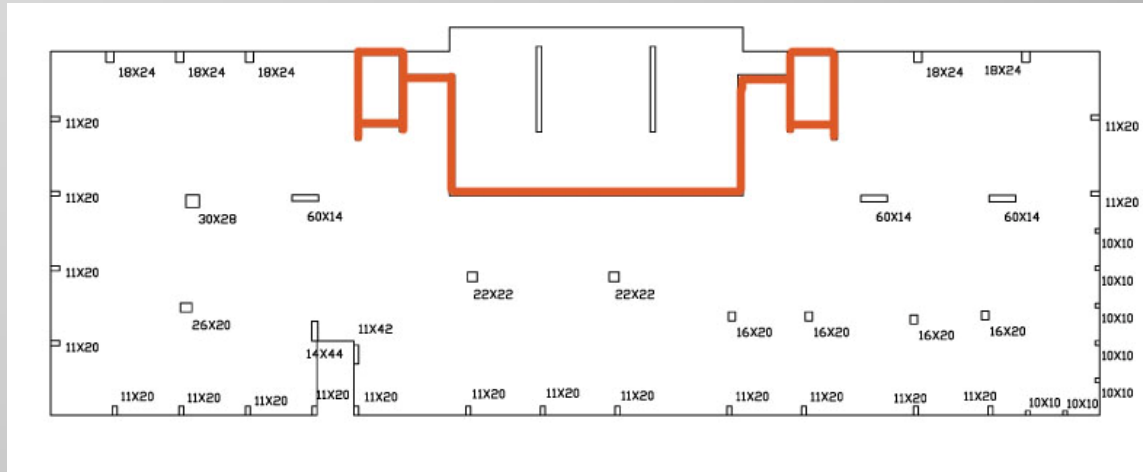
THESIS PRESENTATION

SPRING 2007



# EXISTING STRUCTURE

## Floor Plans



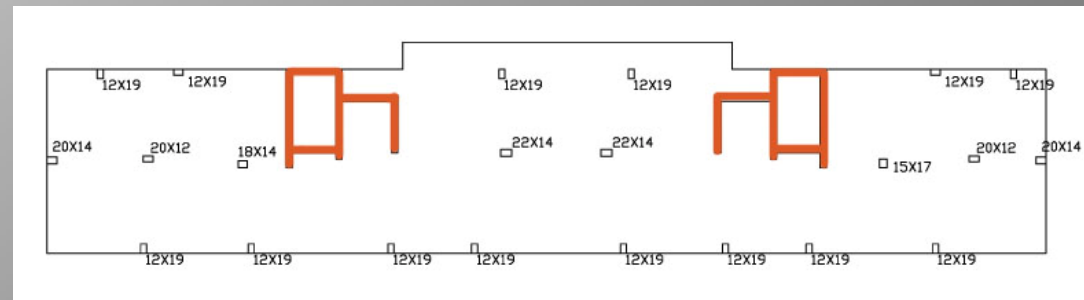
10 1/2" Flat Plate

Uniform Steel

#4 @12" o.c. bot. ea. way

#5@16" o.c. top ea. Way

Significant additional steel  
at column locations

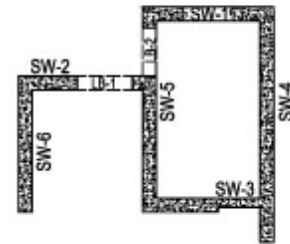
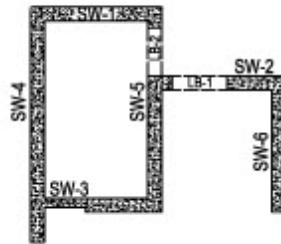


# EXISTING STRUCTURE



Base Thru 5th

6th Thru 13th



Lateral System

EXISTING CONCRETE LINK BEAM SCHEDULE

MARK	SIZE		REINFORCEMENT		STIRRUPS	LOCATION	REMARKS
	D	W	TOP	BOTTOM	SIZE & SPACING		
LB-1	33"	12"	8-#9	8-#9	#4@4"	6TH THRU ROOF	T&B BARS IN 3 LAYERS
LB-2	33"	12"	8-#9	8-#9	#4@4"	6TH THRU ROOF	T&B BARS IN 3 LAYERS
LB-3	106"	12"	10-#9	10-#9	#4@6"	5TH FLOOR ONLY	T&B BARS IN 4 LAYERS
LB-4	44"	12"	10-#9	10-#9	#4@6"	2ND THRU 4TH	T&B BARS IN 4 LAYERS
LB-5	106"	8"	5-#9	5-#9	#4@6"	5TH FLOOR ONLY	T&B BARS IN 2 LAYERS
LB-6	44"	8"	5-#9	5-#9	#4@6"	2ND THRU 4TH	T&B BARS IN 2 LAYERS

# Structural Depth

JOSEPH MUGFORD

THESIS PRESENTATION

SPRING 2007





# STRUCTURAL DEPTH

## LWC FLAT PLATE DESIGN GOALS & STRATEGY

Minimize usage of structural materials to offset the premium of LWC

### Reduce Slab Thickness

1. Deflection equations derived by R.I. Gilbert
2. Add edge beams at building corners to support reduced slab

### Re-Design Shear Wall Core

1. Elements that are more efficient in bending
  - Offset reduced gravity loads
2. Uniform Cross section throughout height of building
  - Consistent formwork
  - Efficient load path with limited number of vertical discontinuities



# STRUCTURAL DEPTH

## Materials

Existing System

New System

Floor Slabs

4 ksi NWC

4 ksi LWC

Shear Walls

5 ksi NWC

5 ksi NWC



# Structural Depth

Flat Plate



# STRUCTURAL DEPTH

## LWC vs. NWC

### BENEFITS

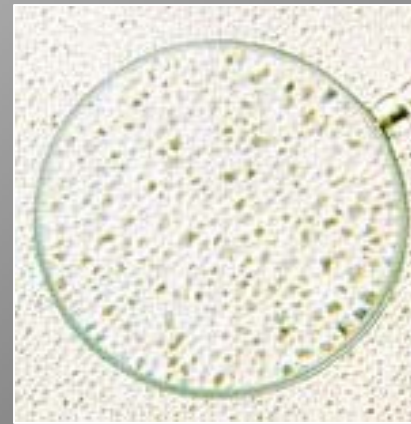
- Better acoustical performance
- Better thermal properties
- Higher fire resistance
- Reduced autogeneous shrinkage
- Improved contact zone between aggregate and concrete matrix
- less micro-cracking as a result of better elastic compatibility
- Higher blast resistance
- Recycled additives (fly ash, etc.)
- **SIGNIFICANT WEIGHT REDUCTION**

- "Structural Lightweight Concrete"

Expanded Shale Clay and Slate Institute

### COSTS

- \$20-30/ Cu. YD premium
- compressive strengths exceeding 4 ksi not available
- Alternative design and detailing to NWC



# STRUCTURAL DEPTH

## DEFLECTION RESEARCH AND APPLICATION

### Gilbert-85

•Modification of Branson Beam Equation

$$\frac{l}{d} \leq K_1 K_2 K_3 \left[ \frac{\Delta}{l} \left( \frac{\alpha b_{eff} E_c}{k(w_v + cw_s)} \right) \right]^{0.33}$$

Annotations for the equation above:  
- 1.3 points to  $K_1$   
- 1.0 points to  $K_2$   
- 1.0 points to  $K_3$   
-  $15\sqrt{pn}$  points to  $\alpha$   
-  $2.0-1.2A_s'/A_s$  points to  $c$

\*sustained loading  
includes 50% live load

= 9" Slab Depth

$K_1$  = Support Factor

$K_2$  = Shape Factor

$K_3$  = Slab System Factor

$\Delta$  = Maximum Deflection

$\alpha$  = Effect of  $\rho$  and  $n$

$b_{eff}$  = Effective Width

$E_c$  = Modulus of Elasticity

$k$  = Load Fraction Carried by Equivalent Beam

$w_v$  = Variable Load

$w_s$  = Sustained Load

$c$  = Long Term Deflection Modification



# STRUCTURAL DEPTH

## DEFLECTION RESEARCH AND APPLICATION

### Gilbert-99

- Modification of current ACI deflection equation
  - $\beta$  accounts for tension cracking resulting from shrinkage and long term loading
  - Computed deflection multiplied by  $\lambda$  per ACI 318 (2.0 for long term loading)

$$I_e = \beta \left( \frac{M_{cr}}{M_a} \right)^3 I_g + \left[ 1 - \beta \left( \frac{M_{cr}}{M_a} \right)^3 \right] I_{cr} \leq I_g$$

0.4 (long term loading)

$$= \ell / 390$$



# STRUCTURAL DEPTH

## COLUMN TRANSFER

### Transfer Forces

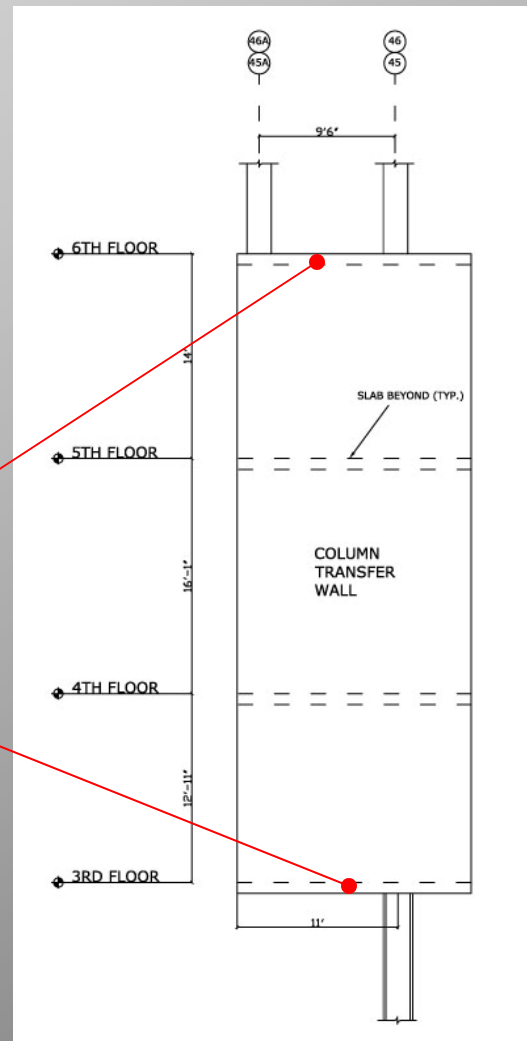
$$\text{Moment} = 5434'k$$

$$\text{Shear} = 253'k$$

### Shear Transfer Reinforcement

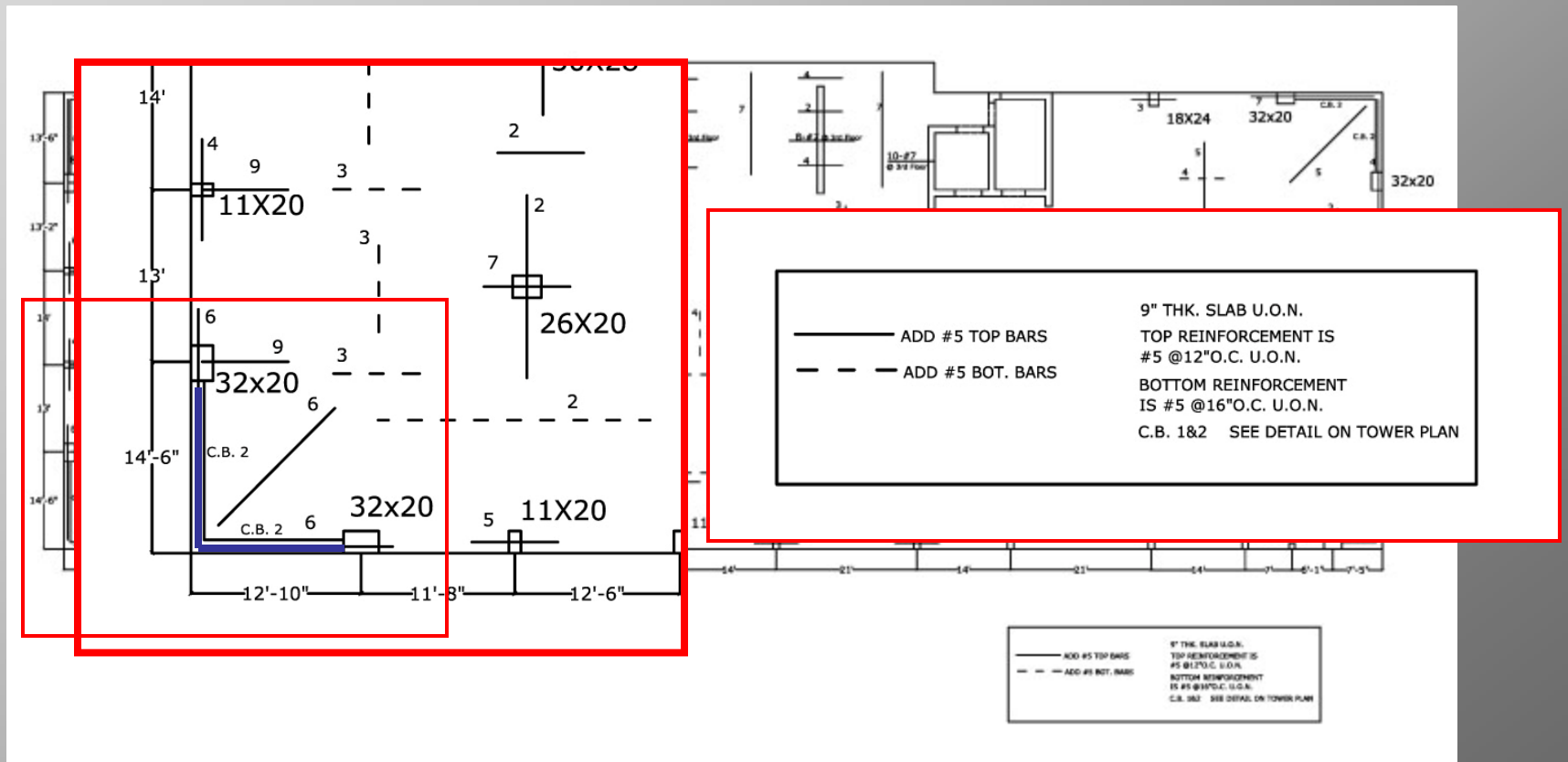
8-#7 @ Transfer walls

10-#7 @ Shear walls



# STRUCTURAL DEPTH

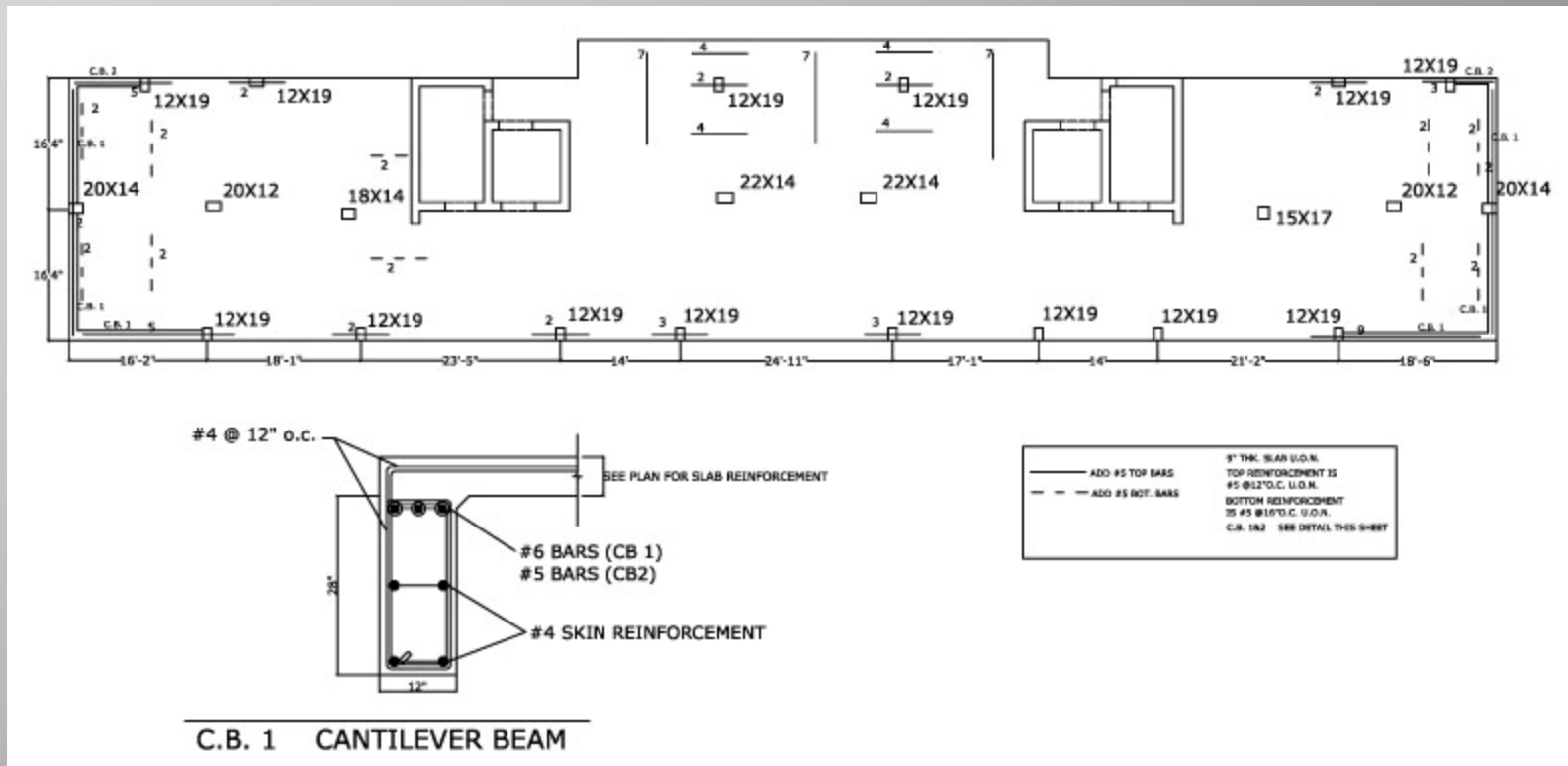
## LWC FLAT PLATE DESIGN





# STRUCTURAL DEPTH

## LWC FLAT PLATE DESIGN



# STRUCTURAL DEPTH

## LWC FLAT PLATE DESIGN

Equivalent Frame Method

Design Slab depth=9"

Uniform Reinforcement

#5@16" o.c. bottom ea. way

#5@12" o.c. top ea. way

Cantilever Beams

12X28 cantilever beams at building corners  
Corner columns increase to 20 x 32 @ Base

Column Transfer

Additional 8-#7 @Gravity Wall  
Additional 10-#7 @Shear Wall



# Structural Depth

## Lateral System

JOSEPH MUGFORD

THESIS PRESENTATION

SPRING 2007



# STRUCTURAL DEPTH

## SHEAR WALL DESIGN CRITERIA

### Lateral forces

Wind

ASCE 7-05 Method 2

Earthquake

ASCE 7-05 Equivalent Lateral Force Procedure (ELF)

Dynamic analysis (ETABS)

Shear From Column Transfer

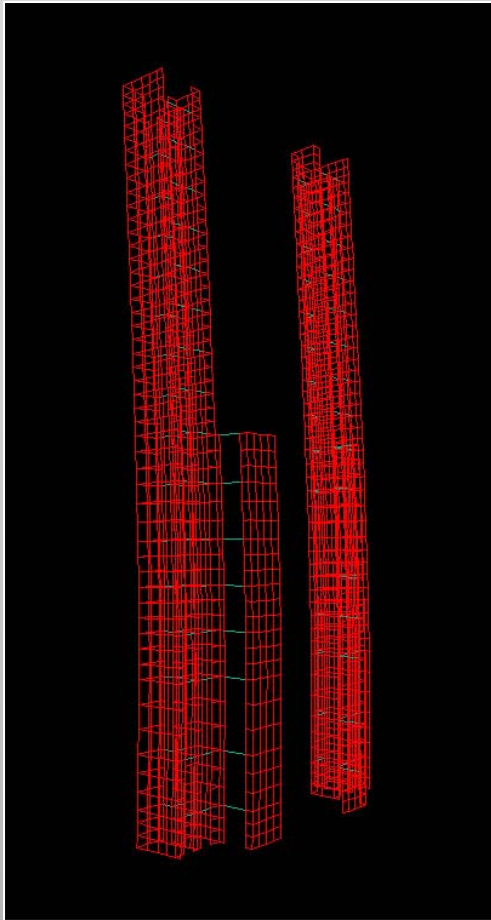
### Deflection

Total Building Drift  $H/500$  (Wind)

Total Building Drift  $H/400$  (Seismic)



# STRUCTURAL DEPTH



## N-S Wind Controls

Level	Load (k)		Shear (k)		Moment (ft-k)	
	N/S	E/W	N/S	E/W	N/S	E/W
Roof	26	5	0	0	4,348	846
12	48	9	26	5	7,455	1,443
11	45	9	75	15	6,382	1,235
10	44	9	120	23	5,712	1,101
9	43	8	165	32	5,038	965
8	43	8	208	40	4,521	866
7	42	8	251	48	3,890	740
6	64	10	293	56	5,200	840
5	79	25	357	67	5,426	1,736
4	79	25	436	92	4,354	1,384
3	61	19	515	117	2,381	745
2	59	18	577	136	1,524	471
1	66	20	636	155	853	256
Totals	702	174	702	174	57,083	12,626

# STRUCTURAL DEPTH

## E-W Seismic Controls

### Modal (Dynamic) Analysis

- Response Spectrum

*IBC 2000 Function scaled to NYC  $S_{DS}$  &  $S_{D1}$*

- Base Shear=386 K (ELF)
- R=4 (Normally Reinforced Shear Walls)

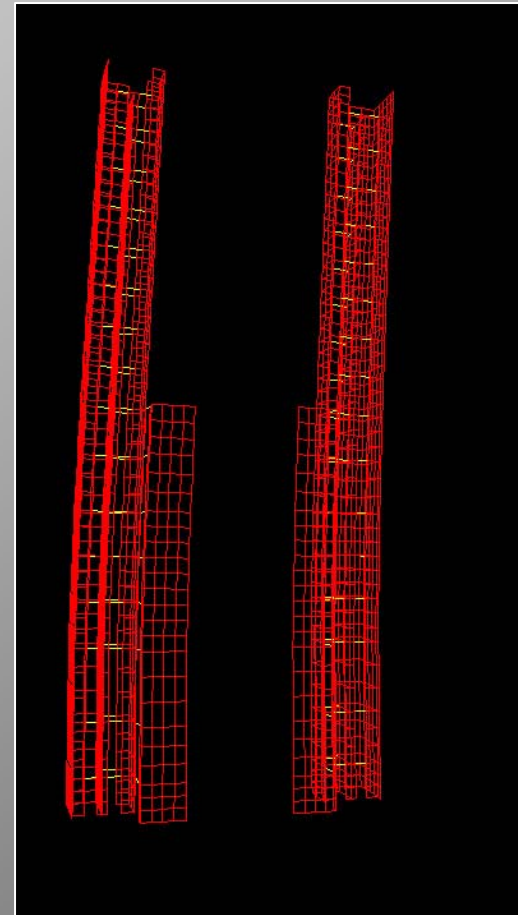
### Element Stiffness

modified to account for cracking of concrete and force distribution

Walls=0.7I<sub>g</sub> (uncracked)

Coupling Beams=0.35I<sub>g</sub>

T=2.06s

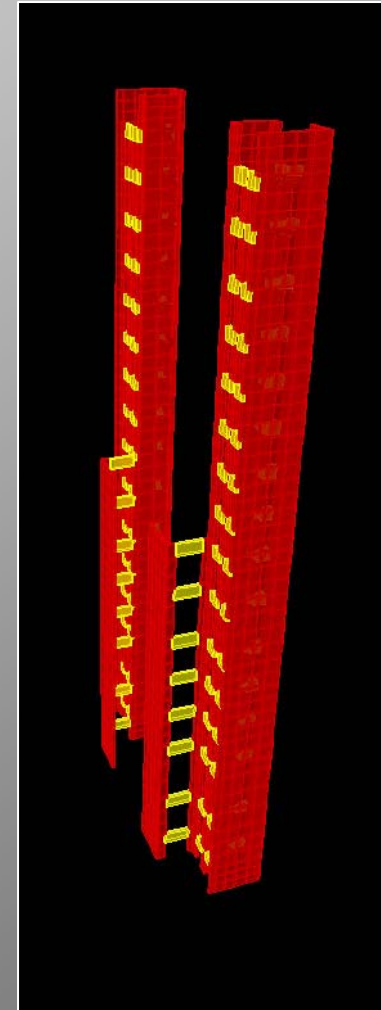


# STRUCTURAL DEPTH

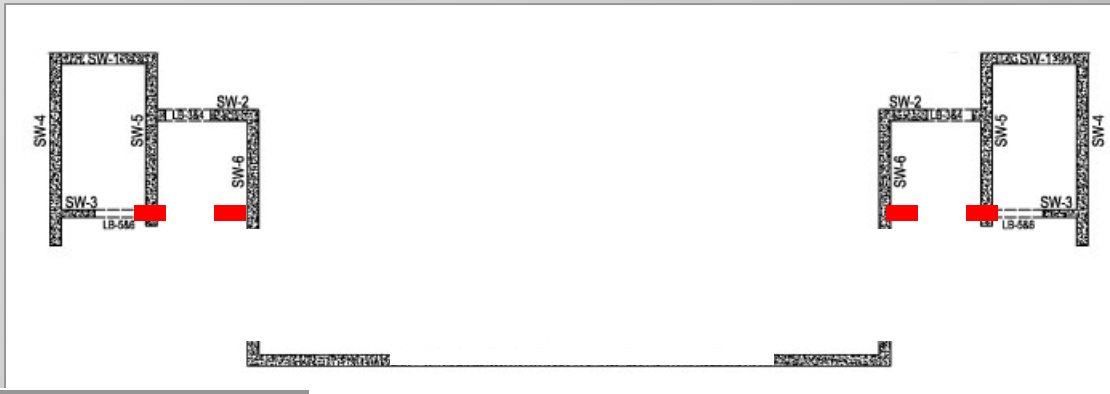
## Story Displacements

Story	Load	$\Delta X$	$\Delta Y$
BH 2	WINDX	0.75	0.00
BH 2	Quake X	3.03	0.13
BH 2	WINDY	0.08	4.04
BH 2	QUAKE Y	0.07	3.50
BH 1	WINDX	0.75	-0.01
BH 1	Quake X	3.03	-0.16
BH 1	WINDY	0.08	3.48
BH 1	QUAKE Y	0.07	2.98
ROOF	WINDX	0.71	0.00
ROOF	Quake X	2.84	0.02
ROOF	WINDY	0.02	3.60
ROOF	QUAKE Y	0.02	3.10
STORY 13	WINDX	0.66	0.00
STORY 13	Quake X	2.62	0.00
STORY 13	WINDY	0.02	3.25
STORY 13	QUAKE Y	0.02	2.79
STORY 12	WINDX	0.60	0.00
STORY 12	Quake X	2.36	0.00
STORY 12	WINDY	0.02	2.90
STORY 12	QUAKE Y	0.01	2.47
STORY 11	WINDX	0.54	0.00
STORY 11	Quake X	2.12	0.00
STORY 11	WINDY	0.01	2.57
STORY 11	QUAKE Y	0.01	2.17
STORY 10	WINDX	0.49	0.00
STORY 10	Quake X	1.87	0.00
STORY 10	WINDY	0.01	2.25
STORY 10	QUAKE Y	0.01	1.88
STORY 9	WINDX	0.43	0.00
STORY 9	Quake X	1.62	0.00
STORY 9	WINDY	0.01	1.93
STORY 9	QUAKE Y	0.01	1.60

Story	Load	$\Delta X$	$\Delta Y$
STORY 8	WINDX	0.37	0.00
STORY 8	Quake X	1.37	0.00
STORY 8	WINDY	0.01	1.62
STORY 8	QUAKE Y	0.01	1.32
STORY 7	WINDX	0.32	0.00
STORY 7	Quake X	1.14	0.00
STORY 7	WINDY	0.01	1.32
STORY 7	QUAKE Y	0.01	1.07
STORY 6	WINDX	0.26	0.00
STORY 6	Quake X	0.89	0.00
STORY 6	WINDY	-0.04	1.04
STORY 6	QUAKE Y	-0.03	0.83
STORY 5	WINDX	0.20	0.00
STORY 5	Quake X	0.65	0.00
STORY 5	WINDY	-0.03	0.74
STORY 5	QUAKE Y	-0.02	0.59
STORY 4	WINDX	0.13	0.00
STORY 4	Quake X	0.40	0.00
STORY 4	WINDY	-0.02	0.45
STORY 4	QUAKE Y	-0.01	0.35
STORY 3	WINDX	0.07	0.00
STORY 3	Quake X	0.23	0.00
STORY 3	WINDY	-0.01	0.25
STORY 3	QUAKE Y	-0.01	0.19
STORY 2	WINDX	0.03	0.00
STORY 2	Quake X	0.09	0.00
STORY 2	WINDY	0.00	0.10
STORY 2	QUAKE Y	0.00	0.07



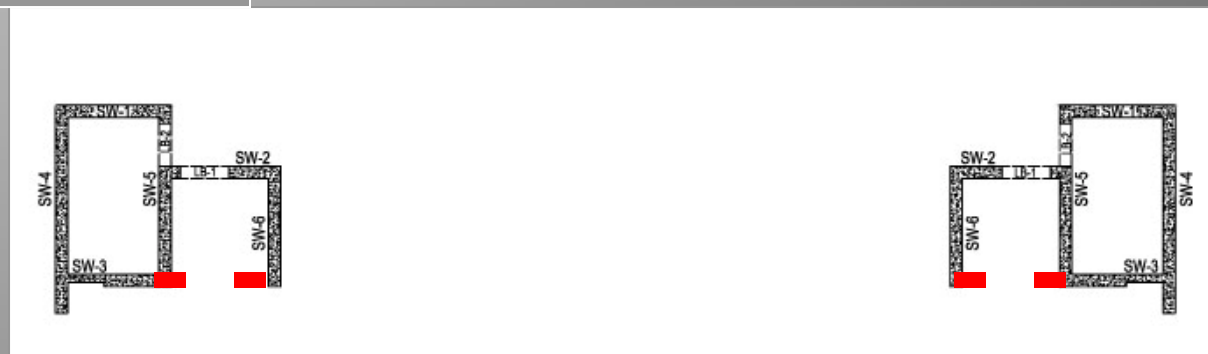
# STRUCTURAL DEPTH



Base Thru 5th

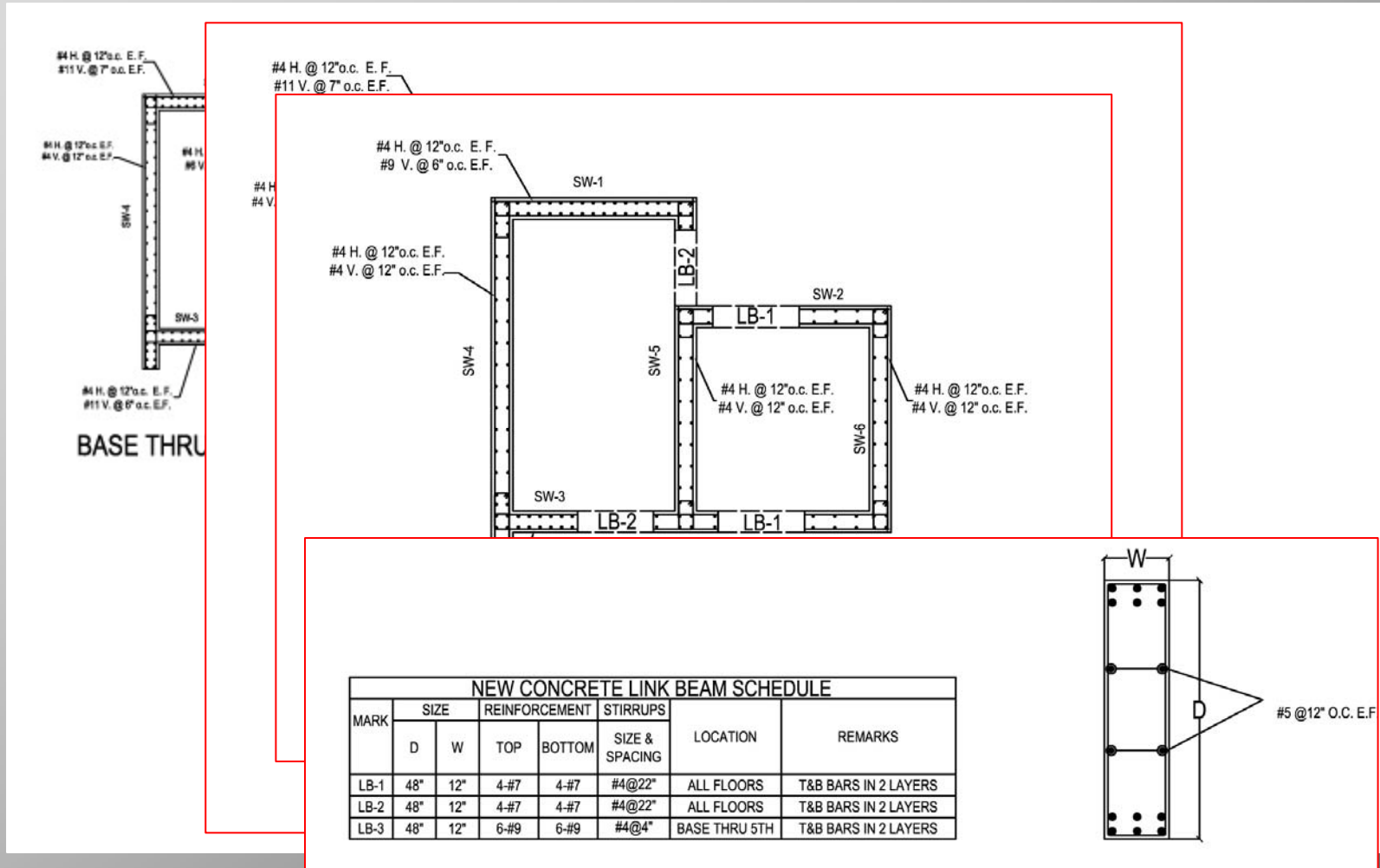
Lateral  
System  
Modification

6th Thru 13th





# STRUCTURAL DEPTH



# LEED Breadth

JOSEPH MUGFORD

THESIS PRESENTATION

SPRING 2007



# LEED BREADTH

*“A green building is one whose construction and lifetime of operation assure the healthiest possible environment while representing the most efficient and least disruptive use of land, water, energy and resources. The optimum design solution is one that effectively emulates all of the natural systems and conditions of the pre-developed site – after development is complete.”*

*-Zeigler*

## Design Goal

Gain LEED Certification of the SOHO High Rise (Implement 26/69 LEED Criteria):

- Acknowledge criteria already met
- Identify additional suitable criteria and associated costs



# LEED BREADTH

## Water Efficiency

Water Efficiency Worksheet		Daily Uses	Flow Rate	Duration	Residents	Water Use	
Flush Fixture			(GPF)	(Flushes)		(GPD)	
	Conventional Water Closet (M & F)	3	1.6	1	132	634	
	Low Flush Water Closet (M & F)	3	1	1	132	396	
Flow Fixture			(GPM)	(Sec)			
	Conventional Lavoratory	3	2.5	12	132	198	
	Conventional Kitchen Sink	1	2.5	12	132	66	
	Conventional Shower	1	2.5	180	132	990	
	Laminar Flow Lavoratory	3	1.8	12	132	143	
	Laminar Flow Kitchen Sink	1	1.8	12	132	48	
	Laminar Flow Shower	1	1.8	180	132	713	
						Conventional Water Use	1888
						Efficient Water Use	1299
Assumptions						%reduction	31
3 occupants per condominium							
44 Total Condominiums							



# LEED BREADTH

## Associated Costs

Credit	LEED Credits	Costs
<b>Sustainable Site</b>		
1-Site Selection	1	\$0
2-Development Density	1	\$0
4.1-Alternative Transportation- Public Transportation	1	\$0
4.2-Alternative Transportation-Bicylce Storage	1	\$505/rack
7.1-Heat Island Effect (Non-Roof)	1	\$0
<b>Water Efficiency</b>		
3.1-Reduced Water Usage- 20%	1	\$0
3.2-Reduced Water Usage-30%	1	\$1,500
<b>Energy and Atmosphere</b>		
1-Increase Energy Performance (15%)	1	\$0
3-Additional Commissioning	1	1-5% Building
4-HCFC and Halon free	1	Significant
<b>Materials and Resources</b>		
2.1-Construction Waste Management, Divert 50% from Disposal	1	\$25/TON
4.1-Recycled Content, 10% (post-consumer + ½ pre-consumer)	1	\$0
5.1-Regional Materials, 20% Extracted, Processed & Manufactured Regionally	1	\$0
7-Certified Wood	1	\$0
<b>Indoor Environmental Quality</b>		
3.1-Construction IAQ Management Plan, During Construction	1	Varies
3.2-Construction IAQ Management Plan, Before Occupancy	1	Varies
4.1-Low-Emitting Materials, Adhesives & Sealants	1	\$0
4.2-Low-Emitting Materials, Paints & Coatings	1	\$0
4.3-Low-Emitting Materials, Carpet Systems	1	\$0
4.4-Low-Emitting Materials, Composite Wood & Agrifiber Products	1	\$0
5-Indoor Chemical & Pollutant Source Control	1	Minimal
7.1-Thermal Comfort, Design	1	\$0
7.2-Thermal Comfort, Permanent monitoring system	1	\$0
8.1-Daylight & Views, Daylight 75% of Spaces	1	\$0
8.2-Daylight & Views, Views for 90% of Spaces	1	\$0
LEED Certified Professional	1	
<b>Credits Attained</b>	<b>26</b>	



# Conclusions and Recommendations



# CONCLUSIONS AND RECOMMENDATIONS

## Structural

- 13-18% Reduction of Column Service Loads ~4500K
- 8-10% Reduction in Slab Reinforcement
- 19 ½" Total Height Reduction
- 25% Reduction of Seismic Base Shear

- Applicable to high seismic regions
- Uniformity of shear wall system
  - Predictable lateral response
  - Consistent formwork

\*Does not include possible column cross section reduction

		Existing	Redesign	Comparison	
		Cost	Cost		
Flat Plate	Base	Concrete	\$228,165	\$232,875	\$4,710
		Rebar	\$283,700	\$259,780	\$23,921
		Cantilevers	\$0	\$33,198	\$33,198
	Tower	Concrete	\$140,175	\$143,125	\$2,950
		Rebar	\$174,335	\$159,615	\$14,720
		Cantilevers	\$0	\$53,116	\$53,116
	Shear Walls	Walls	Concrete	\$109,984	\$121,072
		Rebar	\$64,455	\$119,472	\$55,016
Coupling Beams					
			\$92,039	\$77,428	\$14,611
			Cost Increase		\$106,826

## Architectural

- Elimination of shear wall section provides for architecturally free floor plan
- Lower gravity loads may increase sellable floor area by reducing column cross sections



# CONCLUSIONS AND RECOMMENDATIONS

## LEED

It should be the goal of developers and designers to implement design solutions that not only provide low first cost, but also improve the world and environments we live in.

Implement additional LEED criteria that benefit occupants and meet allowable development budget

Certification carries significant cost and schedule implications

Notable social status symbol





Questions?



AE Faculty

GMS.LLP

SLCE ARCHITECTS

FAMILY & FRIENDS

THANK YOU

